

**POVERTY POINT LIMESTONE  
QUARRY SLOPE STABILITY  
ASSESSMENT AND DESIGN  
GUIDELINES**

Submitted to: HOLCIM (US) Inc.

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**NORWEST**  
CORPORATION

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## 1 INTRODUCTION

### 1.1 TERMS OF REFERENCE

HOLCIM plans an expansion of their limestone and aggregate quarry at Poverty Point. Norwest Corp was engaged by HOLCIM to investigate, evaluate, and report on the stability of rock slopes in the quarry and to make recommendations for future mining. This report lists sources of information, and based on the available data, describes local geology and geologic structure, identifies discontinuity types, rock mass characteristics, and hydrogeological conditions. The report also presents preliminary slope stability assessment results and slope design/mining method guidelines, provides conclusions and makes recommendations to support and verify the stability assessment and slope design. Additional geotechnical information will need to be acquired before design guidelines are finalized.

The site is located 35 miles west of Salt Lake City, Utah near the southwest shore of the Great Salt Lake and at the north end of Skull Valley (see Figure 1.1). The existing quarry was advanced from the southwest to the northeast, generally perpendicular to the strike and up-dip of rock bedding. The highest point of the existing quarry wall is approximately 200' above the quarry floor. The quarry floor is at 4580' and the top of the wall is at 4800'. A plan view of the existing quarry and the proposed final quarry area appears as Figure 1.2. The small summit behind the quarry wall to the northeast has a maximum elevation of 4920'. The quarry has been inactive for several years.

### 1.2 POVERTY POINT SOURCES OF INFORMATION

Information used to describe quarry slope conditions and make design recommendations was gathered from sources including:

- a) Geologic logs for DH-06-01, DH-06-02, DH-06-03, and DH-06-04 as logged by Tom Newman, February 2006 (Appendix A).
- b) Geotechnical Rock Core Logs for DH-06-02, DH-06-03, and DH-06-04 as logged by Eric Martin, February 2006 (Appendix A).
- c) Digital photos of core collected at DH-06-02, DH-06-03, and DH-06-04 (see Appendix B)
- d) Plan view of Poverty Point Quarry as provided by HOLCIM.
- e) Geologic cross-sections A-A', B-B', C-C' showing the interpreted geology in the area as provided by HOLCIM (Appendix C).

- f) June 2005 site visit by Gary Stubblefield, Norwest Corp, Salt Lake City with digital photos and quarry description (Appendix D).
- g) United States Geological Survey data on precipitation amounts and traditional groundwater levels in the area.
- h) Laboratory testing of representative core samples including point load and unconfined compressive strength tests (Appendix E).
- i) Rock mass classification of strata based on geotechnical core logs.

### **1.3 QUARRY DEVELOPMENT AND MINING OPERATIONS**

A final quarry development plan has not been completed; however Norwest understands that the current plan would involve expanding the quarry to advance the pit to the 4920' elevation north of the existing quarry. Mine operators plan to begin mining the existing quarry from the northeast, removing the overlying talus, and advancing southwest, downslope and downdip, mining high purity limestone as ore and underlying siliceous limestone for aggregate.

The operation will also include external waste rock dumps, a crushing plant, and associated infrastructure.

It is assumed that, mining will take place using truck and loader/excavator methods. Drilling and blasting will be carried out to achieve suitable fragmentation. The topography of the quarry area and the quarry configuration allows for access into the quarry beginning at the 4920' elevation and advancing with a maximum highwall defined by the height of each lift (20-50'). There will be no final highwall, as the operation will effectively start at the highest point of the existing topography and work downslope, mining along footwall bedding. An endwall will develop on the northwest end of the quarry. Benching of the footwall may be unnecessary, dependent upon ultimate quarry depths and extents defined by HOLCIM.

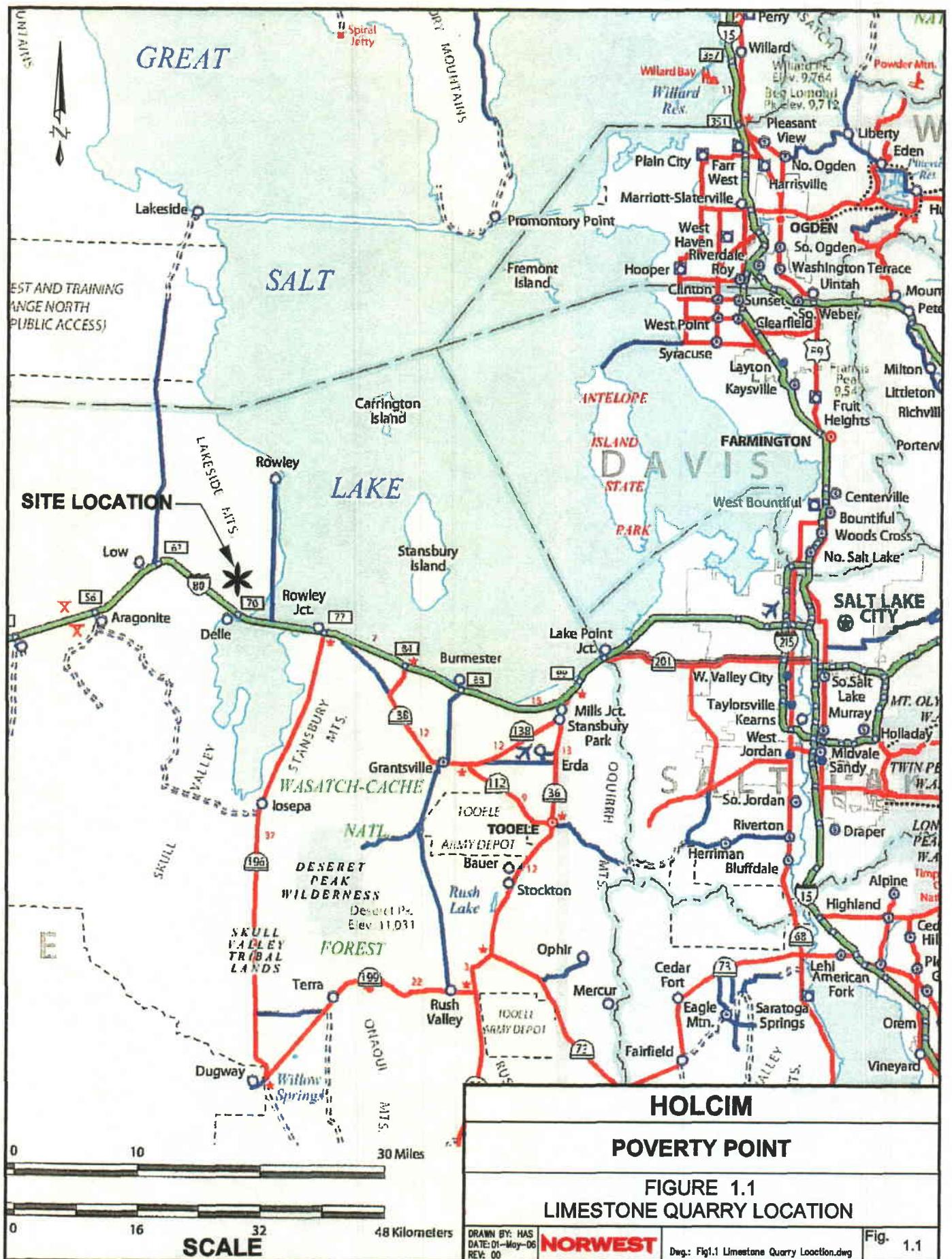


FIGURE 1.1  
LIMESTONE QUARRY LOCATION



0 500 1000 1500

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**HOLCIM**

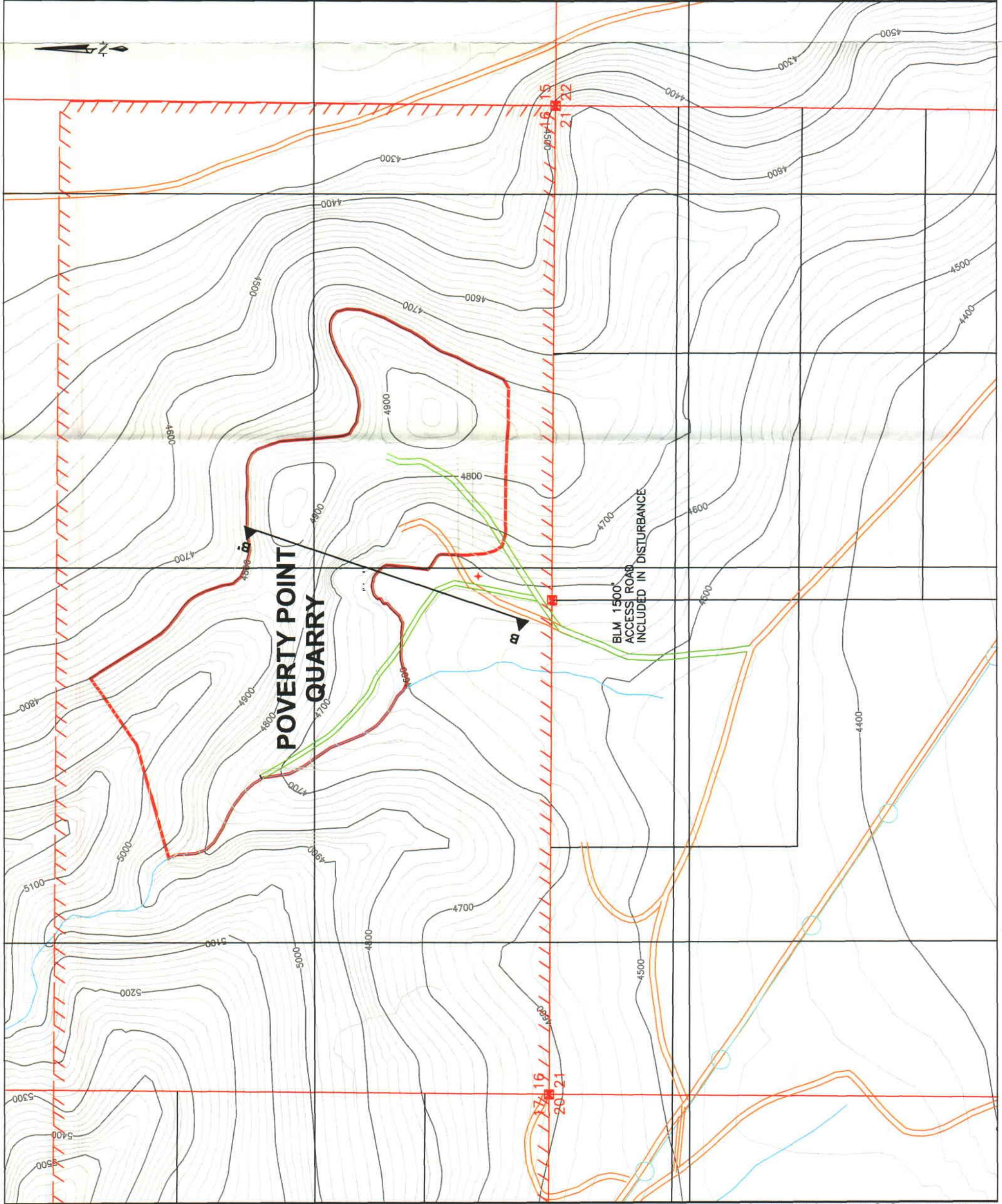
**POVERTY POINT**

FIGURE 1.2  
PLAN VIEW

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Fig. 1.2



## 2 GENERAL GEOLOGY

### 2.1 POVERTY POINT STRATIGRAPHY

Rock units in the area are as follows:

- Great Blue Formation Limestone - Limestone is the predominant rock type at the quarry. HOLCIM geologists have further defined limestone rock units (based upon chemistry) as either high purity or siliceous. All limestone has origins in the Upper and Middle Pennsylvanian (Palaeozoic Belt Supergroup) shallow marine sediments. Limestone beds have thicknesses varying from 5'-75'. Limestones are fine to medium grain, competent rock with medium to dark grey coloration, calcite healed fractures, some chert bands, minor clay partings, stylolites and fossils. Iron staining is also common.
- Great Blue Formation Sandstone – A thin sandstone unit (10') underlies limestone units. Origin is in Upper and Middle Pennsylvanian Palaeozoic Belt Supergroup.
- Cemented Talus – A thick talus unit (20'-75') overlies the limestone in most areas. This unit is composed of well cemented (recemented) Lake Bonneville cobbles and limestone fragments and is commonly identified as a weak conglomerate.
- The surficial material is Quaternary age alluvial deposits of sand and gravel, which overlie the limestone beds at the toe of the existing pit.

A cross-sectional view of quarry stratigraphy developed by HOLCIM geologists and based upon geochemistry appears as Figure 2.1. The cross-section is also shown in plan view on Figure 1.2.

### 2.2 POVERTY POINT STRUCTURE

The structural geology of the Poverty Point area exhibits gently sloping (19-35°) bedded rock units, dipping out of the existing quarry face. Beds strike from N90W to N45W. Bedding planes are daylighted in quarry walls which have overall wall angles up to 35°.

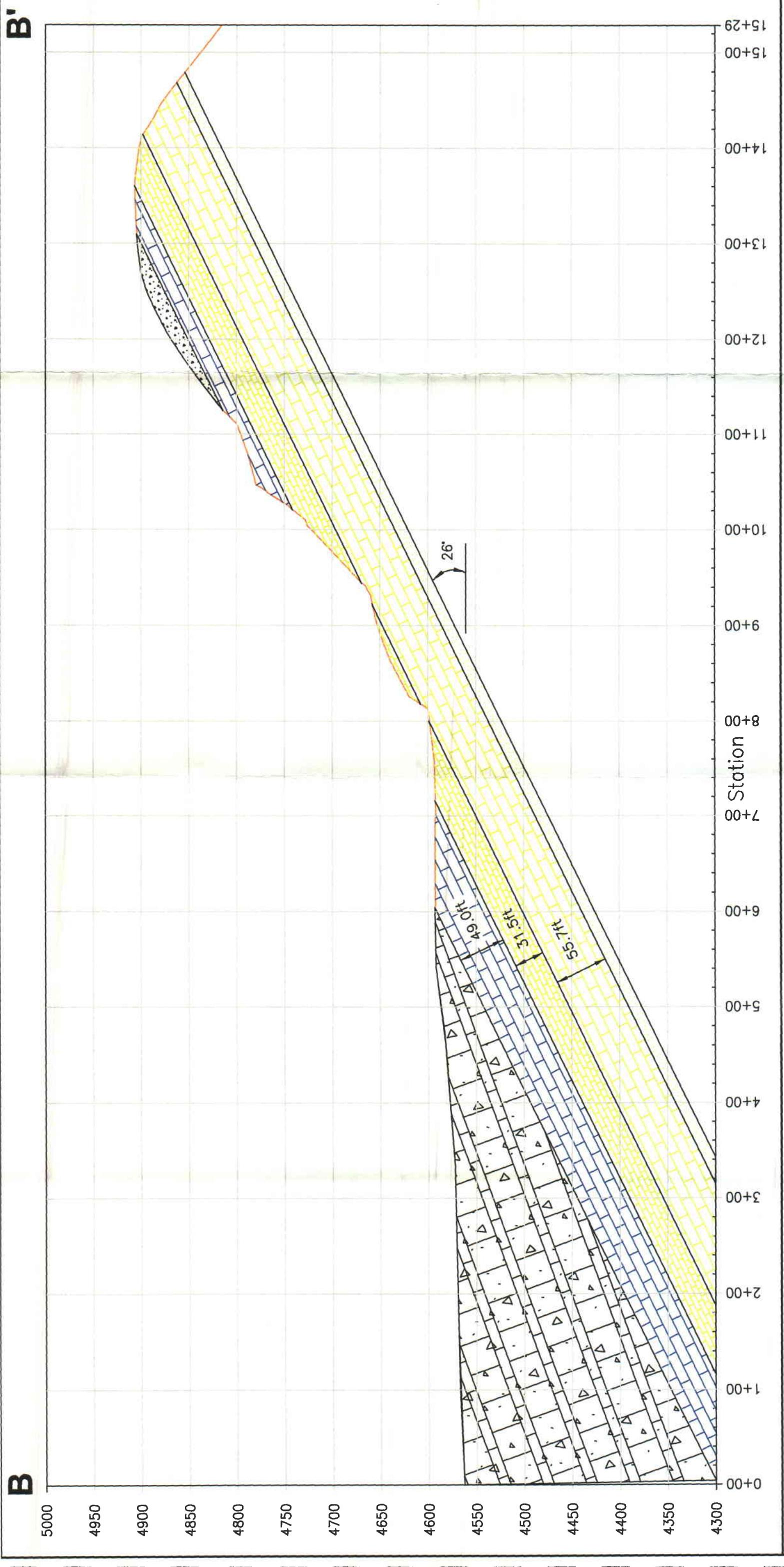
Two fault zones are identified in close proximity to the quarry on plan views provided to Norwest by HOLCIM. One fault passes through the northwest wall of the proposed quarry expansion and strikes N30E, and the other is immediately southeast of the proposed quarry wall and strikes N40E. Field investigators also identified and photographed a fault zone

passing through the middle of the existing quarry. Additional faults are identified both southeast and northwest of the quarry, which have strike orientations ranging from N30E to N50E.

Rock units are generally competent with few throughgoing structures. Discontinuities identified within drill logs were characterized as planar with both rough and smooth surfaces and calcite infilling. Iron oxide staining was also observed. Fractures were mapped with orientations of N22E, N33E and N40W.

Location of faults and orientation mapping of bedded rock units are shown in plan view in Figure 2.2.

During initial start-up efforts at the Poverty Point Quarry, additional geotechnical investigation and analysis should be completed to further identify and describe discontinuities (joint orientations, infilling, presence and character of faulting etc.) and to account for their potential effect on slope stability.

**B'**

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FIGURE 2.1 STRATIGRAPHY		Dwg.: Pit Sections_20060426.dwg	Fig. 2.1
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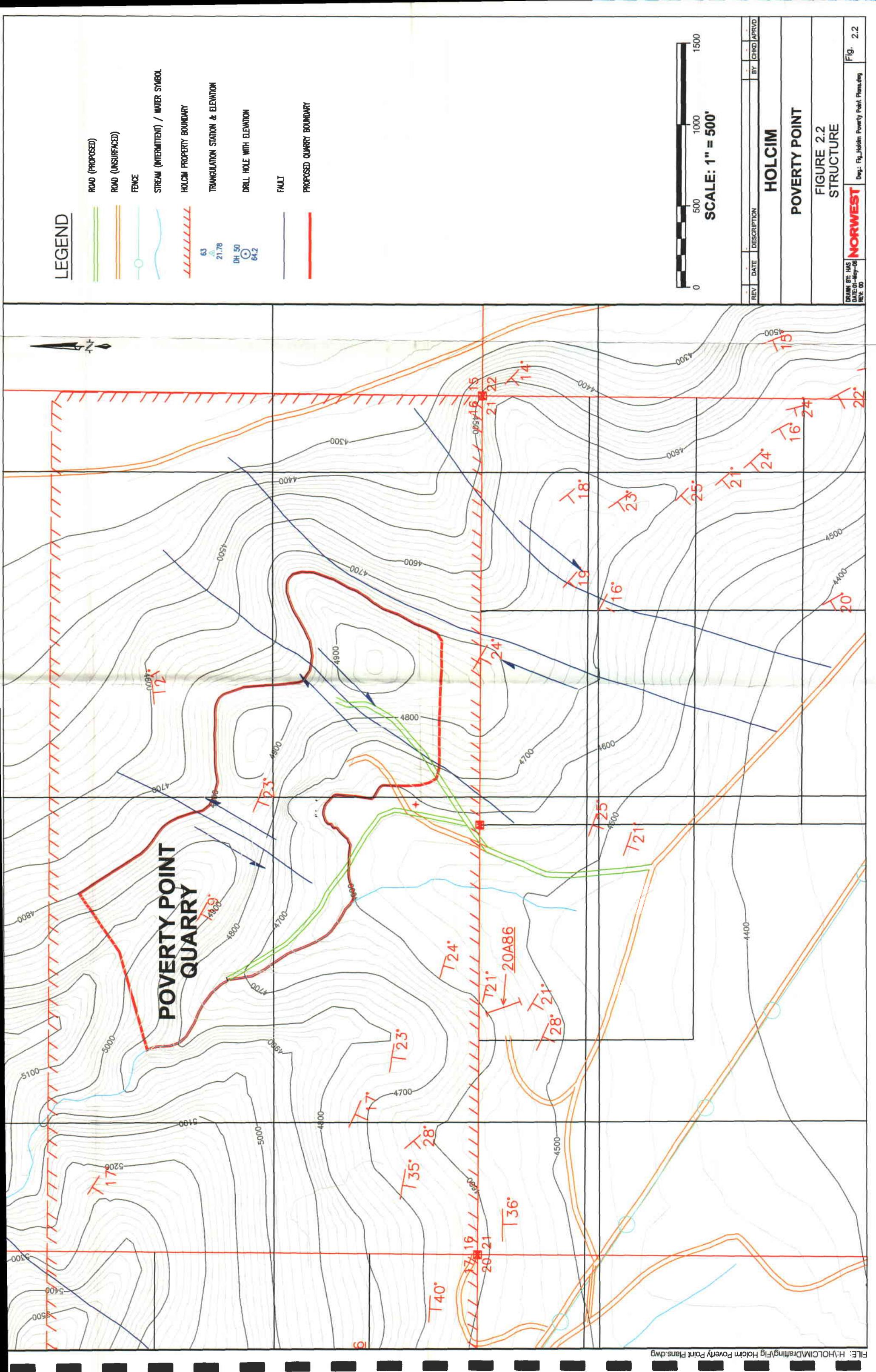
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### 3 ENGINEERING GEOLOGY

This report section describes the engineering geology of the Poverty Point Quarry. In particular, it includes a discussion of the rock joint characteristics and a presentation of the measured and calculated rock mass properties as means for estimating shear strength.

Rational design in bedded slopes is best carried out by assigning engineering properties according to various mappable lithological units. Thus the three main lithologies in the area, limestone, sandstone, and talus (conglomerate) form useful groupings for engineering purposes even though fairly wide variations and gradations can occur within and between these different lithologies.

#### 3.1 ROCK JOINT CHARACTERISTICS

##### 3.1.1 Bedding Joints

Single bedding joints within mappable rock units typically are very persistent, extending for several hundred feet with spacing varying from 1-10'. Due to flexural slip, polished and slickensided bedding surfaces may be common at lithological contacts. It is expected that the spacing, strength, and persistence of bedding will be the major controlling factor for footwall quarry slope stability, in the absence of adversely oriented faulting.

The shear strength of bedding joints is best assessed by considering the weakest bedding planes, as these will control the stability of slopes. In the absence of site specific data, bedding friction angles were based upon typical values for each rock type (see Table 1) for use in preliminary stability evaluation. Site specific values ← should be confirmed as operations restart.

TABLE 1  
DESIGN BEDDING FRICTION ANGLES

Bedding Unit	Friction Angle*
Limestone	30°
Talus (Conglomerate)	30°
Sandstone	25°

\*Estimated from Hoek and Bray, 1981

### 3.1.2 Cross Joints

Minor cross joints have been identified in the existing Poverty Point Quarry face, but systematic mapping has not been carried out, so the degree of orientation scatter and frequency of subsets is not known. Core logging and photos indicate that cross joints are uncommon, but do occur perpendicular to bedding at spacing of 1-20' and are relatively discontinuous. Cross joints are not confined to individual bedding units. Extensive mapping of structural orientations has not been carried out and bench face structure mapping should be undertaken during mining to verify and support design assumptions.

### 3.1.3 Faults and Shears

Current geological data and interpretation has identified two fault structures which could be mined out and should therefore not influence overall final slope stability. A third fault may be exposed in the existing pit face. No thrust fault or other shear-type structures have been identified. It is likely that some minor structures will occur as mining progresses which may affect overall pit wall stability. Ongoing mapping during operations is necessary to detect adverse structures, and measures for managing resulting slope issues need to be included in the operational plan.

## 3.2 ROCK MASS CHARACTERISTICS

Rock mass properties were determined from drill hole cores taken from the February 2006 drilling program. Figure 3.1 shows drill hole locations. Representative core samples were chosen for point load and unconfined compression testing. Results are shown in Table 2. Point load and unconfined compression test results fall within the range of generally accepted values for similar rock types.

TABLE 2  
SUMMARY OF LABORATORY STRENGTH TESTING RESULTS

Rock Type	Point Load (Tensile) Strength (MPa)			Unconfined Compressive Strength (MPa)			Range of Typical Values (MPa)	
	Max	Min	Average	Max	Min	Average	Point Load*	UCS**
Limestone	11.4	4.2	7.9	125.8	53.0	84.0	4.0-11.0	50-100

\*Adapted from Lama and Vutukuri, 1978

\*\*Hoek and Bray, 1981

Rock mass rating (RMR) classifications for rock units were developed using test data from Table 2 and geotechnical core logging data (Rock Quality Designation (RQD), joint spacing, and joint condition) from drillholes in the existing quarry area. The RMR system provides a means for estimating rock mass shear strengths (Hoek, 1994). For the purpose of calculating RMR, water conditions were estimated as damp. RMR results are summarized in Table 3. RMR calculation results are provided in Appendix F.

**TABLE 3**  
**SUMMARY OF ROCK MASS RATING CALCULATION RESULTS**

Rock Type	Rock Mass Rating			RMR System Description
	Max	Min	Average	
Limestone	77	30	55	Fair Rock

Unconfined compressive strength (UCS) values indicate that the limestone which comprises the majority of the quarry wall is moderately strong rock and the RMR system confirms this impression based upon point load, core data, and field observations.

The data from Tables 2 and 3 were used to generate rock mass strength parameters using guidelines set forth by Hoek (1994). Mi values (a constant which defines the relationship between principal stresses at the failure point for a given rock) were estimated from a standardized table (Hoek and Karzulovic, 2000). Shear strength envelopes were generated for the limestone rock units within the quarry walls. Shear envelopes were generated using disturbance factors of 0 and 1. Disturbance factors of 0 correlate to rock masses with no significant seismic impacts affecting their structural integrity. Disturbance factors of 1 correlate to rock masses exposed to some seismic impacts (such as that generated by drilling and blasting). Table 4 shows the values used to generate rock mass shear strength envelopes, which in turn are used to characterize rock mass strengths for stability analysis. Shear strength envelope data and curves appear in Appendix G of this report.

**TABLE 4**  
**ROCK MASS STRENGTH PROPERTIES FOR STABILITY ANALYSIS**

Rock Type	RMR	MI	UCS (MPa)
Limestone	55	9	80

Additional investigation and analyses are required to characterize the rock mass strengths for the other rock units (sandstone and talus/conglomerate). Drilling in 2006 did not intersect either of these units, either due to drill hole location or drillhole depth. Future investigations should be designed to target these rock units, drill to the elevation of the ultimate quarry floor, complete geologic and geotechnical logs, and obtain and test samples of rock core.

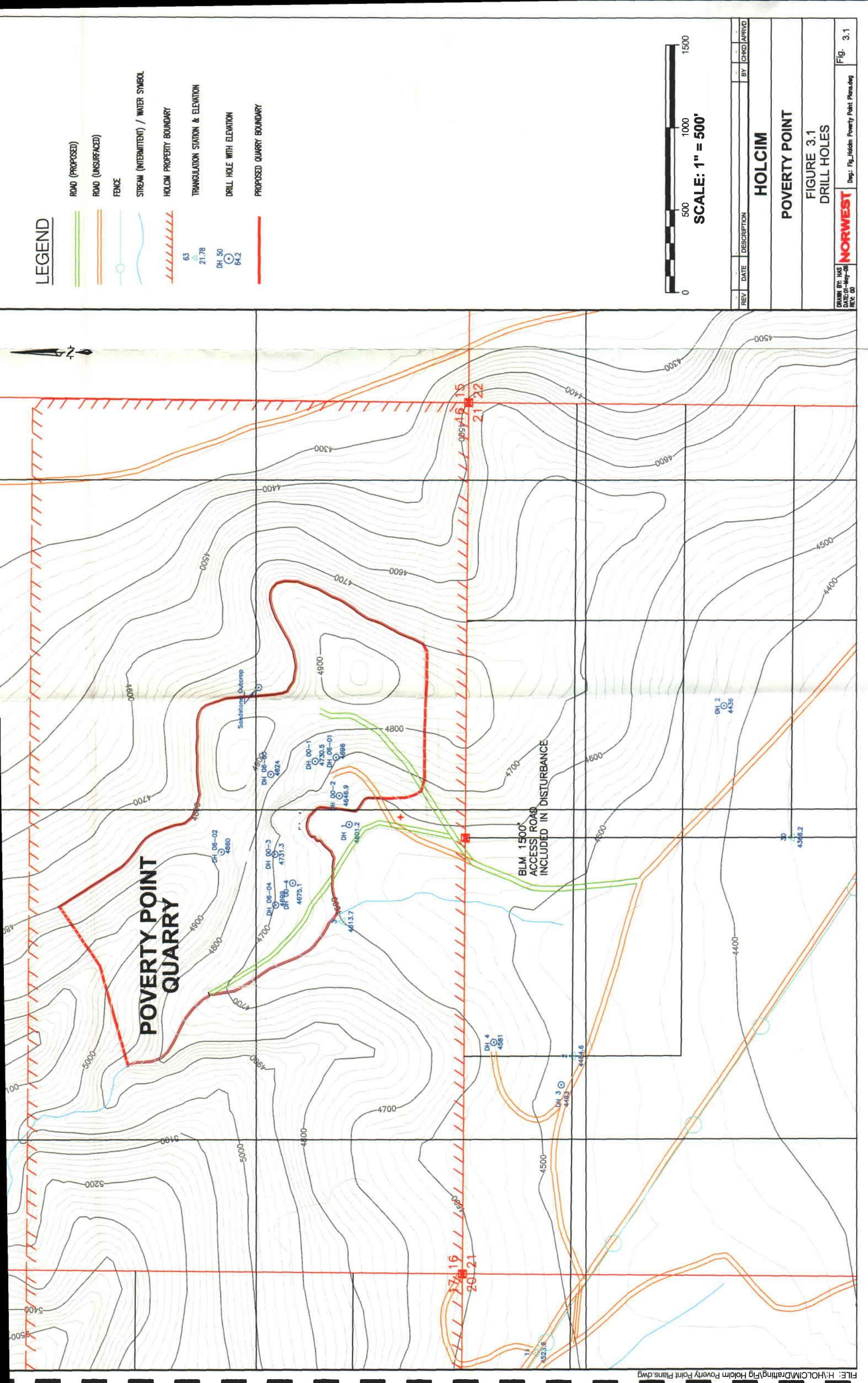
### **3.3 HYDROGEOLOGY**

Limited hydrogeologic data is available for the Poverty Point Quarry; however some qualitative field information can be evaluated and compared to knowledge of groundwater conditions at monitoring sites in similar strata with similar climatological conditions.

The quarry is located on the western edge of the Great Salt Lake Desert with average annual precipitation of 10" (United States Geological Survey). Higher elevations above the quarry may receive as much as 15" annually.

As bedding in the area dips to the southwest, into the existing quarry and away from the planned expansion, water inflow should move out of the mining area through natural seepage and outflow. It is likely all rock units have low hydraulic conductivity, however, flow along bedding planes or discontinuities may be higher. A minor seep was identified on the upper portion of the pit face and small accumulations of water were also noted at the base of the quarry.

Water is not expected to play a critical role in determination of slope stability, as quarry conditions are generally dry. As average groundwater levels have not been reliably established, if future investigations identify water behind pit slopes, its effect upon the integrity of rock units should be accounted for in stability assessment.



## 4 SLOPE STABILITY ASSESSMENT

This portion of the report discusses potential failure modes and provides stability assessments for existing quarry slopes, and proposed footwall and endwall slopes. This assessment assumes that, no bench faces, other than the working face, will be developed on footwall slopes.

### 4.1 EXISTING QUARRY

Quarrying was carried out at Poverty Point prior to the planned expansion addressed in this report. The previous operation developed a quarry advancing from the southwest to the northeast. Rock unit bedding planes strike perpendicular to the direction of mine advance and dip 20-35° southwest. Mining daylighted a number of large bedding planes varying in thickness from 1-50'. These rock units are now retained solely by frictional force and bedding roughness. As it is assumed that the friction angle of bedding units is 30°, in areas where the bedding angles approach 30°, slopes could be at or near a factor of safety (FOS) of 1.0. It is understood that HOLCIM geologists have identified tension cracks above and behind the highwall, which confirms that these slopes are quasi-stable.

### 4.2 FOOTWALL SLOPE

#### 4.2.1 Potential Failure Mechanisms

There are four primary failure modes that typically govern the design of bedded footwall slopes:

1. Planar slab failure – Daylighted failure planes in steep footwall slopes are obvious stability problems. The existing quarry is a prime example of this failure mode, as daylighted bedded units have become unstable and tension cracks have been identified near the crest of the quarry.
2. Bilinear slab failure – In the absence of daylighted bedding planes, this is the primary failure mechanism considered for stability analyses of the footwall. This mechanism involves sliding parallel to bedding in combination with sliding along a secondary surface dipping out of the slope (Dawson and Barron, 1989). The secondary surface may be a persistent discontinuity surface or, if unfavourably oriented discontinuities are not present, a shear zone (slip surface) developed through the rock mass.
3. Ploughing slab failure – Based on the geologic structure of the quarry, this failure mode is unlikely to occur. No thrust structures that should lead to ploughing were identified at the Poverty Point Quarry.

4. Buckling failure – no contributing factors such as high axial pressures, large curvatures in bedding, or high water pressures were identified. These conditions could be present and slope stability assessments should monitor for them.

#### **4.2.2 Assessment**

The unbenched footwall slopes are to be developed on competent rock units ( $RMR > 55$ ), at relatively low angles, and in the absence of daylighting planes of weakness, it is expected that the full height of the quarry footwall may be developed unbenched (up to a maximum wall height of 300'). Measures for controlling the unbenched slopes are discussed in Sections 5 and 6.

Bilinear slab failures have been identified as the primary failure mode in the footwall. Based on current information, variables which may contribute to bilinear failure include:

- slab thickness,
- slab height,
- ground water pressure,
- bedding angle, and
- critical slope angle.

All of these variables were incorporated into the slope stability analysis to determine factors of safety for quarry slopes to bilinear planar failure. Slope/W software was used with fully defined slip surfaces to mimic critical bilinear slab geometries. This software uses the method of slices to calculate the limit equilibrium factor of safety, effectively comparing driving forces to resisting forces. A range of angles for slip surfaces (failure planes) through the rock mass toe were applied. An example of this modeling is shown in Figure 4.1. Slope/W plots appear as Appendix H.

Slope/W analysis was conducted to identify the maximum height of quarry slopes for bedding angles ranging from 20-35° while maintaining a minimum factor of safety of 1.2. A plot of the results is included in Figure 4.1. Results are also reported in Table 5.

**TABLE 5**  
**MAXIMUM UNBENCHING FOOTWALL HEIGHTS @ FOS 1.2**

Bedding Angle	Footwall Height (ft)
20°	2600
25°	1450
30°	740
35°	485

Results of this initial analysis determined that for the range of bedding angles identified at Poverty Point, footwall slopes may be developed to heights exceeding 400' while maintaining a factor of safety of 1.2.

Slope/W analysis was conducted to identify factors of safety for a range of bedding angles (dip of inclined beds was varied from 20-35°) and a range of slope heights. A sensitivity analysis was also conducted to determine the effect of blast damage (FOS for Disturbed Limestone). Table 6 shows analyses results with FOS for the range of slope/bedding angles, and for undisturbed and disturbed conditions. All wall heights were modeled at 300' with no ground water table.

**TABLE 6**  
**UNBENCHING FOOTWALL FACTORS OF SAFETY**

Bedding Angle	Undisturbed Limestone FOS	Disturbed Limestone FOS
20°	4.56	2.63
25°	3.42	1.96
30°	2.83	1.64
35°	2.10	1.22

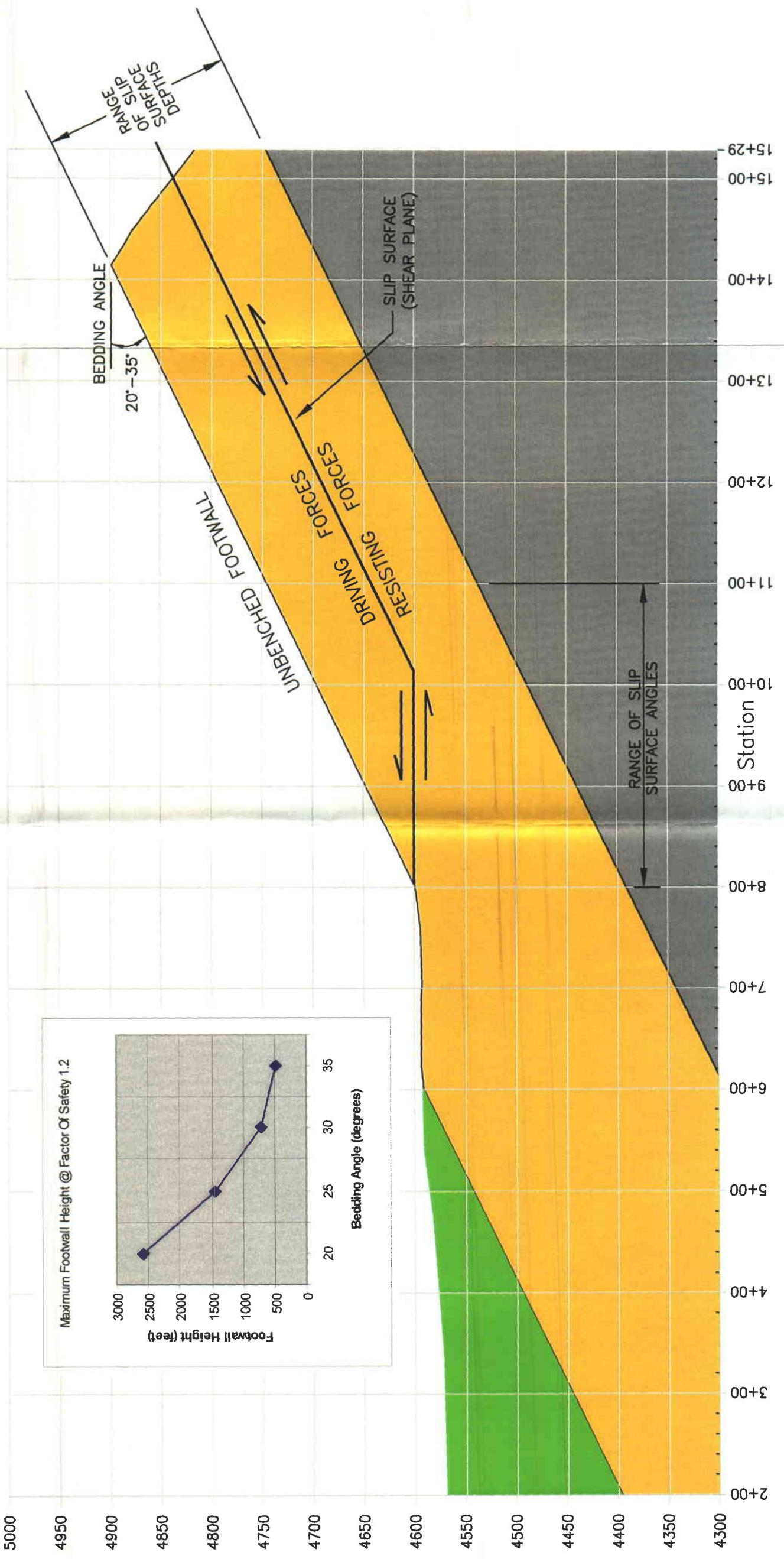
As the high factors of safety indicate, the limestone units behave as competent rock when subjected to stability analysis. Bedding angles up to 35°, with the footwall in limestone disturbed by blasting, 300' slope heights, and dry conditions will support unbanching footwall slopes. It is reasonable to assume that mine operators should be able to develop the mine from the northeast quarry rim, along unbanching footwall to the bottom of the existing quarry.

Other small-scale slope stability events such as rock ravelling may occur on footwall slopes and should be controlled by operational practices including wall scaling, berthing, toe cleanup, trenching, and catch fences as needed.

#### **4.3 ENDWALL SLOPES**

The configuration of the Poverty Point Quarry is expected to produce endwalls formed at one or both ends of the quarry. The endwall slopes are relatively narrow and will cut almost perpendicular to the bedding strike, while bedding dips across the endwall. As a result, significant instabilities are unlikely to occur in these areas. As mining progresses, bench face mapping should be carried out to identify potential for planar, wedge, or toppling failures and to provide additional data for geotechnical evaluation. If mining were to advance southwest, while alternately expanding the extent of the working pit face to the northwest and southeast, the development of endwall slopes could be limited. Ultimately, some endwall slope area will be developed in the northwest quarry. Bench heights should be matched to equipment capabilities and regulation berm widths.

# BILINEAR SLAB FAILURE CONFIGURATION



## LEGEND



REV	DATE	DESCRIPTION	BY	CHKD	APRVD

**HOLCIM**

**POVERTY POINT**

**NORWEST**

**STABILITY ASSESSMENT**

SCALE: 1" = 100'

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FIGURE 4.1  
STABILITY ASSESSMENT

## 5 PRELIMINARY DESIGN GUIDELINES

This section provides preliminary design guidelines for mining method and slope designs of the Poverty Point Quarry.

### 5.1 MINING METHOD ON FOOTWALL SLOPES

Figure 5.1 shows the recommended mining method and footwall design. For footwall slopes the following preliminary design guidelines apply:

- Mining is advanced starting at approximate 4920' elevation and moving downslope using 20-50' lifts. Lift size will be dictated by the size of loading equipment used. The general direction of mining is from the northeast to the southwest.
- Footwall slopes are developed parallel to bedding to minimize waste extraction and prevent undercutting and daylighting potentially unstable strata. Overall slope angle varies with bench face angle (20-35°).
- The footwall follows competent strata (sandstone or limestone unit) for the maximum height allowable (up to 300' for 35° bedding angles). .
- If bench face mapping identifies significant discontinuities which could contribute to slope instability, benching will be carried out to comply with equipment capabilities. Site specific design widths should be developed but for initial purposes, the general guideline for bench widths as supplied by the SME Mining Engineering Handbook is as follows:

$$\text{Minimum bench width(ft)} = 4.5(\text{ft}) + 0.2 \times \text{bench height(ft)}$$

A minimum 4ft high berm should be placed on the edge of each bench.

Additional comments:

- It is expected that the Poverty Point Quarry slopes will be dry. If significant water (saturated rock or flowing water) is encountered as mining progresses, additional investigation and remediation should be considered.
- Blast designs are carried out to maintain setback distance from the footwall to limit damage and maintain a continuous bedding plane. Buffer blasting or trim blasting should be considered along trim rows to limit blast damage.

## 5.2 ENDWALL SLOPES

As endwall slopes are developed, the following preliminary design guidelines are recommended:

- Endwall slopes are developed in accordance with equipment capabilities. Bench face angle is determined by bench face pit mapping and subsequent slope analysis.
- Width of catchment benches for initial design follows general guidelines for bench widths as supplied by the SME Mining Engineering Handbook:

$$\text{Minimum bench width(ft)} = 4.5(\text{ft}) + 0.2 \times \text{bench height(ft)}$$

A minimum 4ft high berm should be placed on the edge of each bench. Bench face pit mapping and subsequent slope analysis may necessitate increasing bench widths to provide for bench scale wedge or toppling failures.

- For overburden and weathered rock slopes, all unconsolidated material shall be sloped to an angle less than the angle of natural repose. In the case of the weathered rock at the Poverty Point Quarry, all bench face angles shall be sloped to <45°.
- Blast designs are carried out to maintain bench face angle. Buffer blasting or trim blasting is carried out along trim rows to limit blast damage.

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**HOLCIM**

**POVERTY POINT**

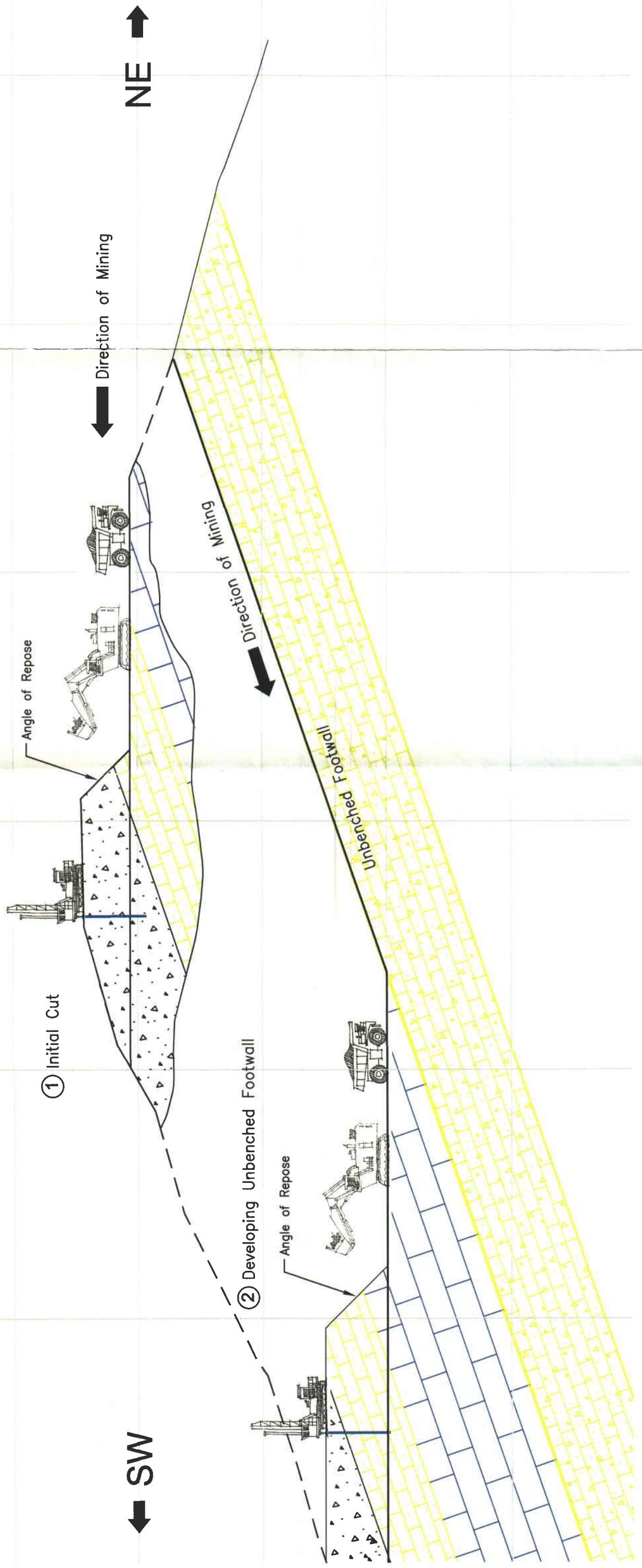
**FIGURE 5.1**

**MINING METHOD**

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Dwg.: Fig5 Mining Method.dwg  
Fig. 5.1

## CONCEPTUAL VIEW OF MINING METHOD



LEGEND	
ALUVIUM	
TALUS	
HIGH PURITY Ls	
SILICEOUS Ls	
SANDSTONE	

## **6 CONCLUSIONS/RECOMMENDATIONS**

This report section presents conclusions and recommendations for slope stability at the Poverty Point Quarry. Unless otherwise specified in this report and until superseded by additional slope stability assessment, all operations should be carried out in compliance with MSHA 30CFR Standards.

### **6.1 CONCLUSIONS**

Having evaluated existing geotechnical information for the Poverty Point Quarry, the following conclusions are reached:

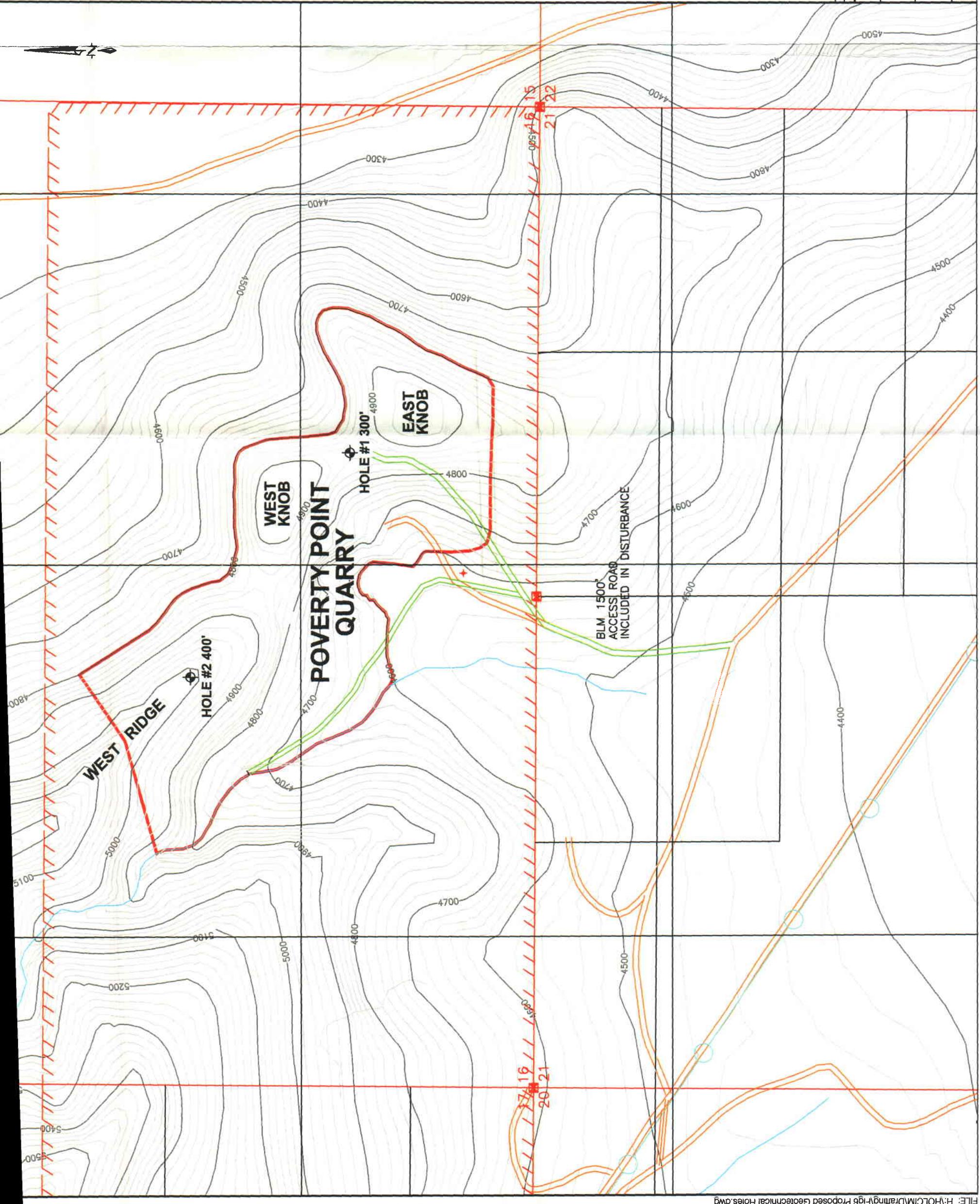
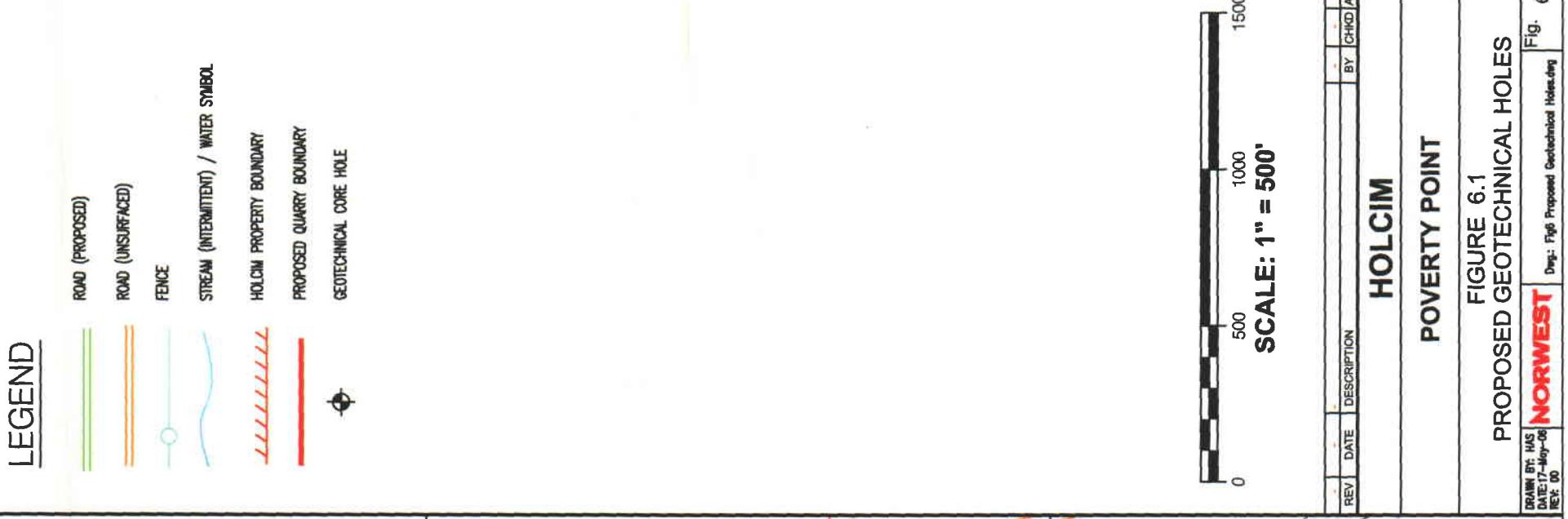
- The practice of mining up dip of bedding units dipping at or near bedding unit friction angles is technically challenging and increases the potential for wall instability. This practice requires detailed investigation of rock strength properties, extensive slope monitoring, and periodic remediation to limit the potential for slope failure.
- The previous mining method at the Poverty Point Quarry daylighted bedding units dipping into the quarry and contributed to quasi-stable conditions.
- Additional geotechnical investigation should be completed prior to mine start-up to verify design assumptions, define additional geotechnical parameters (see section 6.2) and complete a thorough assessment of slope stability at the Poverty Point Quarry.
- Based upon current knowledge of geotechnical parameters at Poverty Point, the quarry may be developed on unbenched footwall slopes and benched endwalls with mining proceeding downslope and downdip of bedding from the northeast to the southwest.

### **6.2 RECOMMENDATIONS**

These recommendations are made to support and verify preliminary design guidelines and to complete the assessment of quarry slope stability:

1. The previous practice of mining up dip should be discontinued. Alternative mining methods are available to HOLCIM operators and should be adopted.
2. Due to the presence of daylighted beds, dipping near or at friction angles, and tension cracks indicating slope movement, the existing quarry floor should be maintained as a restricted access site, signage installed and the area bermed off to prevent access by personnel or mine equipment.
3. Additional geotechnical investigation should be completed, including:

- a) Geotechnical core drilling – 2 vertical holes targeted to capture the full extent of footwall strata – the first hole from 4900' to 4600' at the saddle between the east and west knobs, and the second from 5000' on the crest of the west ridge to the 4600' elevation (see Figure 6.1). Depth to water should be identified at each hole.
  - b) Quarry bench face structure mapping – mapping of strikes, dips, persistence, aperture, surface roughness, water/flow, and other observed structural features.
  - c) Point load and unconfined compressive strength testing of samples from geotechnical core drilling.
  - d) Direct shear testing of samples from geotechnical core drilling to identify friction angles.
  - e) Further site investigation – geologic mapping of surface outcrops, identification of hydrogeologic features (seeps, springs, streams).
4. A slope maintenance and monitoring plan should be developed to define procedures which provide for operations, maintenance, and surveillance of quarry slopes. The plan should address cleaning and/or clearing pit walls and benches, construction of catch berms or trenches where necessary, restricted access provisions, and active mitigation of pit walls. The monitoring section should include slope audit procedures, bench face mapping procedures, emergency preparedness plans, and survey slope monitoring. The plan should also include a basic water management plan with descriptions of water audits, identification of water sources, any necessary construction and maintenance of surface run-off diversion measures, filling and grading of tension cracks to minimize water inflow and other mitigation.
  5. A controlled blasting plan should be developed to minimize damage to the quarry walls. The plan should include blast audits, fragmentation requirements, contingency plans for overblasting, blasthole parameters, measures to monitor and control hole quality, timing sequence, powder factors and kilocalories per ton, stemming, decking, vibration monitoring and control for vibration frequency and shot duration. Alternate plans for blasts adjacent to final walls should be included.



## **APPENDIX A**

### **Drill Hole Logs**

## Geologist's Drilling Report

Holcim

Hole NO. DH-06-1

## GEOLOGIC LOG OF DRILL HOLE

Project POVERTY POINT 2006

L.s. Thickness \_\_\_\_\_

Location POVERTY POINT, UT.

O.B. Thickness \_\_\_\_\_

Coordinates \_\_\_\_\_

Date Started 1-30-06

Collar Elevation N40°48'48.8" W012°45'01.1"-4730

Date Finished 2-2-06

Hole Logged 2-2-06

Driller \_\_\_\_\_

Bearing and Dip VERT

Water Level NONE

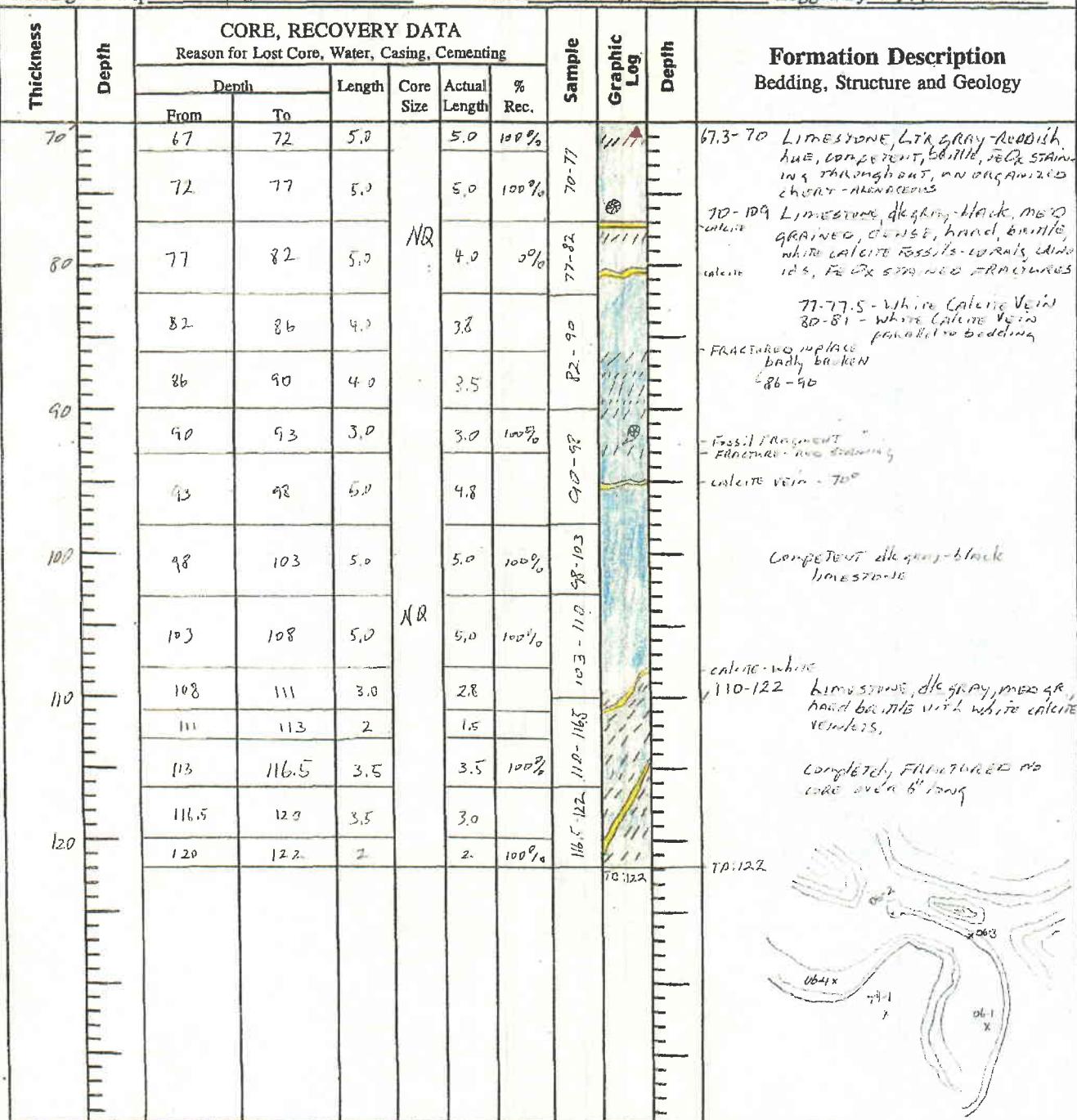
Logged By TIVEMAN

Thickness	Depth	CORE, RECOVERY DATA						Sample	Graphic Log	Depth	Formation Description Bedding, Structure and Geology				
		Reason for Lost Core, Water, Casing, Cementing													
		Depth		Length	Core Size	Actual Length	% Rec.								
From	To														
0	7	7	N/A	0	0	0	0%	0-7	gravel	0-7	OVERBANKS - Light gray-green, black, brownish gravels, some carbonaceous siltstone, slope wash, none above, calcite coatings				
7	13	6		0.5	8%	1.5	10%	7-13		7-28	LIMESTONE, back, brownish tan, very fine grained, dense, tan, light gray bristle, no fossils				
13	18	5		0.5	10%	1.5	20%	13-18			brown, aggregate, thin bedded				
18	20.5	2.5		.5	20%	.5	20%	18-22.5							
20.5	22.5	2.5		.5	20%	.5	20%	22.5-26							
22.5	28	5.5	NQ	1.0	20%	2.5	20%	28-30							
28	34	6.0		5.5	92%	5.5	100%	30-34							
34	39	5.0		5.1	100%	5.1	100%	39-43							
39	43	4.0		3.0	75%	3.0	75%	43-47			- White calcite veinlet down center of core				
43	47	4.0		3.0	75%	3.0	75%	47-52			-47- FRACTURE at 52°				
47	52	5.0	NQ	5.0	100%	5.0	100%	52-57			grading to medium grained				
52	57	5.0		5.0	100%	5.0	100%	57-62			- 55- black chert nodules - hotel bristle 55.1 near summary in fracture				
57	62	5.0		5.0	100%	5.0	100%	62-67.3			- 59- FRACTURE - rock scattering				
62	67	5.0		5.0	100%	5.0	100%	67.3-72			- fossil - white calcite conat good competent limestone				
67	72	5.0		5.0	100%	5.0	100%				- 65- white gunge in fracture plane				
											- 67.3 - IV LIMESTONE, light gray, reddish hue, red grainers, competent, hard SEE p. 2				

Hole Size NQ'

Hole No. DH-06-1

Project Poverty Point 2006 L.s. Thickness \_\_\_\_\_ Top L.s. Elev. \_\_\_\_\_  
 Location Poverty Point, UT O.B. Thickness \_\_\_\_\_ Bottom L.s. Elev. \_\_\_\_\_  
 Coordinates \_\_\_\_\_ Date Started 1-30-06 Total Depth of Hole 122  
 Collar Elevation \_\_\_\_\_ Date Finished 2-2-06 Contractor 6ddbe Drilling  
 Bearing and Dip VERT Hole Logged 2-2-06 Driller \_\_\_\_\_  
 Water Level NONE Logged By T NEWMAN

Hole Size NQHole No. DH-06-1

Project Poverty Point 2006 L.S. Thickness \_\_\_\_\_ Top L.S. Elev. \_\_\_\_\_  
 Location Poverty Point, UT O.B. Thickness \_\_\_\_\_ Bottom L.S. Elev. \_\_\_\_\_  
 Coordinates \_\_\_\_\_ Date Started 2-3-06 Total Depth of Hole 138  
 N  $40^{\circ} 48' 55.6''$  W  $112^{\circ} 45' 08.7''$  Date Finished 2-4-06 Contractor Goddle Drilling  
 Collar Elevation \_\_\_\_\_ Hole Logged 2-4-06 Driller \_\_\_\_\_  
 Bearing and Dip VERT Water Level NONE Logged By T. NEWMAN

Thickness	Depth	CORE, RECOVERY DATA						Formation Description Bedding, Structure and Geology	
		Reason for Lost Core, Water, Casing, Cementing		Sample	Graphic Log	Depth			
		Depth	Length						
From	To	Length	Actual Length	% Rec.					
0	3	3	45%	2.2	73%	0	0-15.2	LIMESTONE, dk. black, mottled, bedded, COMPETENT, IRREGULAR, white CALCIATE STALS, Boulders	
3	6	3	20%	2.0	67%	3	15.2-28.7	2.8F - 2.8 depth, ANASTOMOSING CALCIATE STALS	
6	8	2	80%	2	100%	6	28.7-30.5	5.0F - gray, very calcite crusted, SOFT	
8	11	3	4%	2	67%	11	30.5-33.3	7.0F - locally broken, irregular limestone 12-13.5 IRREGULAR limestone bedded with CALCITE VENGETES	
11	16.5	5.5	36%	2.1	38%	16.5	15.2-60.0	-15.2F 60° white calcite in fracture plane -16.0F 60° -17.5, 17.9F	
16.5	21	4.5	100%	5	100%	21	19.5-20.1	-19.5F - fully dolomized 55° -20.1F - CALCITE	
21	26	5	90%	5	100%	26	22.0-23.3	-22.0F-20.5F, 21.5-22.3 LIMESTONE, dk gray, 23.1-20.5F STALS, grayish brown, mottled, grainy, irregular, white calcite STALS, 23.3-36.9 LIMESTONE, GRAY-BROWN, MOTTLED, MUD GRAINED, COMPETENT, white OPALITE VENGETES	
26	31	5	100%	5	100%	31	36.9-42.4	-29.2F -33.2-42.4 LIMESTONE 15° shallow -36.9F - MARL - CLAY - FEDEX	
31	36	5	96%	5	100%	36	36.9-42.4	36.9-42.4 LIMESTONE, dk gray, mottled, grained, competent, calcite STALS	
36	39	3	0%	2.5	83%	39	42.4-43.0	-42.4-43 Boxwork texture - dissolution, PORES	
39	43	4	40%	3.4	85%	43	43-47	-44.0F DRAKE CLAY GARGE -45.0F	
43	47	4	45%	4	100%	47	47.1-55.6	-47.1F 70° bedding or gray color, mottled, COARSE GRAINED, HIGHLY FRACURATED 47.1-55.6 31.0 CLAY	
47	52	5	20%	4	100%	52	55.6-62	31.0 CLAY	
52	57	5	23%	2.1	100%	57	62-72	31.0 CLAY	
57	62	5	21%	5	100%	62	62-72	41.7-61.5-72° 40° 62.0F-72° - OPALITE beddy broken vertically down the middle of the core at 72°	
62	67	5	61%	5	100%	67	72-77	61.5F 70° 72° - solid competent lime stone 77F - H-10 core AND FEDEX STAINING	
67	72	5	48%	5	100%				

Hole Size NR/VVX R.D Min. 4"Hole No. DH-06-2

Project Poverty Point 2006

L.s. Thickness \_\_\_\_\_

Top L.s. Elev. \_\_\_\_\_

Location Poverty Point, UT

O.B. Thickness \_\_\_\_\_

Bottom L.s. Elev. \_\_\_\_\_

Coordinates \_\_\_\_\_

Date Started 2-3-06Total Depth of Hole 138Date Finished 2-4-06Contractor Globe Drilling

Collar Elevation \_\_\_\_\_

Hole Logged 2-4-06

Driller \_\_\_\_\_

Bearing and Dip VERTWater Level NoneLogged By J Newman

Thickness	Depth	CORE, RECOVERY DATA						Formation Description Bedding, Structure and Geology	
		Reason for Lost Core, Water, Casing, Cementing		Length	MANUFACTURED R.D.	Actual Length	% Rec.		
		From	To						
70'	67	72	5	46/50 96%	5	5	100%	62-72	
				36/50					
	72	77	5	72%	5	5	100%	72-77.5	
				22/50					
80	77	81.5	4.5	44%	4.5	4.5	100%	81.5-87	
				20/50					
	81.5	86	4.5	40%	4.5	4.5	100%	86-92	
				21/50					
86	86	87	1	0%	1	1	100%	87-92	
				21/50					
90	87	92	5	42%	5	5	100%	92-97	
				11/50					
	92	97	5	22%	5	5	100%	97-102	
				0/50					
100	97	100	3	0%	3	3	100%	100-103	
				0/50					
	100	103	3	0%	3	3	100%	103-108	
				6/50					
	103	108	5	1%	5	5	100%	108-113	
				12/50					
110	108	113	5	26%	5	5	100%	113-118	
				10/50					
	113	118	5	20%	5	5	100%	118-121	
				16/50					
120	118	121	3	2%	3	3	100%	121-125.5	
				12/45					
	121	125.5	4.5	27%	4.5	4.5	100%	125.5-130.5	
				22/50					
130	125.5	130.5	5.0	44%	4.8	4.8	96%	130.5-135	
				12/45					
	130.5	135	4.5	26%	4.5	4.5	100%	135-138	
				11/50					
135	135	138	3	25%	3	3	100%	138	
				11/50					

## Geologist's Drilling Report



Hole NO. DH-06-3

## GEOLOGIC LOG OF DRILL HOLE

Sheet 1 of 2

Project Poverty Point 2006 L.s. Thickness \_\_\_\_\_ Top L.s. Elev. \_\_\_\_\_  
 Location Poverty Point, UT O.B. Thickness \_\_\_\_\_ Bottom L.s. Elev. \_\_\_\_\_  
 Coordinates \_\_\_\_\_ Date Started 2-5-06 Sunday Total Depth of Hole 90  
N 40° 48' 52.7 W 112° 45' 02.5 Date Finished 2-6-06 Contractor Bidde Drilling  
 Collar Elevation \_\_\_\_\_ Hole Logged 2-6-06 Driller \_\_\_\_\_  
 Bearing and Dip VERT Water Level NONE Logged By J NEWMAN

Thickness	Depth	CORE, RECOVERY DATA					Sample	Graphic Log	Depth	Formation Description Bedding, Structure and Geology
		Reason for Lost Core, Water, Casing, Cementing								
		Depth	Length	RQD	Actual Length	% Rec.				
From	To									
0	1.5	0-15'	1.5	0%	.5	-				0-35' LIMESTONE, dk gray-black meso to fine grainings, hard brittle, highly fractured, white calcite staining and fracture fillings
	4	0-40'	3.5	0%	2	50%				40-10' - FRACTURED broken rock
	8	4	0%	4	100%					10-25' 58° 10-11.1F 68° SOLID COMPETENT rock
	10	2	0-20'	0%	1.0	50%				CONCRETE grainings
	15	5	39/40		5	100%				14.7F 32° 15.5-16.5' - thin bedded banding
	20	5	50/50		5	100%				16.5F 4 100% REC FECS STAINING
	25	5	100%		5	100%				19.5F 70°
	30	5	45/50		5	100%				21.5F 70° STYLOLITE AT 21.6
	35	5	90%		5	100%				-23.1F 40° CORE - 400 FEET STRAINING
	40	5	74%		5	100%				-24.6F 55° SOLID COMPETENT LIMESTONE
	45	5	45/50		5	100%				-26.3F 25° 26.0 Fossil 400 29.5 VARIOUS TEXTURE
	50	5	96%		5	100%				-29.7F 30° - REC STAINED
	55	5	96%		5	100%				-36.4F 68°
	60	5	48/50		5	100%				-34.5F 70° WHITE CALCIUM FILLED
	65	5	94%		5	100%				-35.0F 41 RUSTIC 40% STAINING
	70	5	34/50		5	100%				-36.2F 40° GRAY-BLACK, EASY SP. DULL COLOR TO BLEMISH
	75	5	68%		5	100%				-37.5F 45°
	80	5	21/50		5	100%				-39.5F 50° CALCIUM 400 400
	85	5	62%		5	100%				-43.4-46 FRACTURED ROCK REBROKEN WITH WHITE CALCIUM, OPEN CAVITIES II TO CORE
	90	5	47/50		5	100%				-47.5-70° LIMESTONE, dk gr-black meso grainings, 48% stylites, hard, compact, white calcite veins, FRACTURE PLANS, bedding planes, CALCIUM VEINS
	95	5	94%		5	100%				-52.6F 41 FLOC STAINING OPEN
	100	5	34/50		5	100%				-54.1F 40° CORE REC FECS STAINED FRACTURE
	105	5	68%		5	100%				-55.2F 40°
	110	5	21/50		5	100%				-58.5F 50° MEDIAL SPACES
	115	5	62%		5	100%				-59.7F 72°
	120	5	47/50		5	100%				-60.1F 40° -600-650M FLOC STAINING
	125	5	94%		5	100%				-61.6F 75° II TO CORE!
	130	5	34/50		5	100%				-63.5F 75° CALCIUM FILLED
	135	5	68%		5	100%				-65.7F 100 FLOC STAINING
	140	5	21/50		5	100%				INTERBEDDING WITH BROWN CHESTS? OR INSOLUBLES
	145	5	62%		5	100%				-69.7F 350 FEET

Hole Size NQ ARD min. 4"Hole No. DH-06-3

## Geologist's Drilling Report



Hole NO. DH-03-3

## GEOLOGIC LOG OF DRILL HOLE

Project Poverty Point 2006 L.s. Thickness \_\_\_\_\_  
 Location Poverty Point, UT O.B. Thickness \_\_\_\_\_  
 Coordinates \_\_\_\_\_ Date Started 2-5-06 Total Depth of Hole 90  
 Date Finished 2-6-06 Contractor bobbe Drilling  
 Collar Elevation \_\_\_\_\_ Hole Logged 2-6-06 Driller \_\_\_\_\_  
 Bearing and Dip VERT Water Level N.D.E Logged By T.N. Newman

Thickness	Depth	CORE, RECOVERY DATA						Formation Description Bedding, Structure and Geology	
		Reason for Lost Core, Water, Casing, Cementing				Sample	Graphic Log		
		Depth	Length	modified R.Q.D.	Actual Length				
From	To								
70	75	5	45/50	5	100%	65-75	131	47.5 - 70.0 LIMESTONE dk gray, blck, mto -73.5F 70° GRANULAR, HARD COMPONENT, layered, IRONSTAINED - CALCITE FILLS, bedding PLANES - O.C., FeOx staining	
75	80	5	50/50	5	100%	75-81	131	71.75 - 70.0 - 81.0 LIMESTONE dk gray, brownish tint, FINE GRAINED, HARD, IRONSTAINED GRANULAR TO O.F., CALCITE FILLS IRONSTAINED, IR DENDRITES	
80	85	5	24/50	5	100%	81-85	131	81-85 BROKEN LIMESTONE dk gray, blck, mto 83.0F 65° IR DENDRITES, IR DENDRITES	
85	90	5	25/50	5	100%	90	131	85 - 85.50 LIMESTONE, dk gray, mto with black cherts - IR DENDRITES, IR DENDRITES IRONSTAINED, IR DENDRITES	
						85-95		88.7F 25° FeOx staining	
100									
110									

Hole Size 1 1/2Hole No. DH-06-3

## Geologist's Drilling Report

Holcim

Hole NO. DH-06-4

## GEOLOGIC LOG OF DRILL HOLE

Project Poverty Point 2006 L.s. Thickness \_\_\_\_\_  
 Location Poverty Point, UT O.B. Thickness \_\_\_\_\_  
 Coordinates \_\_\_\_\_ Date Started 2-6-06  
N40° 48' 52.2 W0102° 3' W112° 45' 42" S Date Finished 2-7-06  
 Collar Elevation \_\_\_\_\_ Hole Logged 2-7-06  
 Bearing and Dip VERT Water Level NONE Logged By J NEWMAN

Sheet 1 of 2

Top L.s. Elev. \_\_\_\_\_

Bottom L.s. Elev. \_\_\_\_\_

Total Depth of Hole 90.5Contractor Bobbe Drilling

Driller \_\_\_\_\_

Thickness	Depth	CORE, RECOVERY DATA					Sample	Graphic Log	Depth	Formation Description Bedding, Structure and Geology
		Reason for Lost Core, Water, Casing, Cementing								
		Depth	Length	Actual Length	% Rec.					
From	To									
0	5	5	52%	3.5	1		D-1.5			Logger wet 38.6 ft recovered
5	10	5	84%	4.5	90%		1.5 - 10			0-1.5 LIMESTONE, GRAY, FINE GRAINED, COMPETENT 1.5 - 8.7 LIMESTONE, GRAY, BLACK, MED GR COMPETENT, FAULTED, FRACUTRED WITH CALCITE FILLING
10	15	5	50%	5	100%					8.0 FT FAULT, SWING 20°
15	16.5	1.5	50%	1.5	100%					3 FAULTS AND SAME CHANNEL
16.5	21.5	5	32%	5	100%		10 - 21.5			-13.7F 20° FLDX 3 FAULTS 18.0F 45° 19.0F 70°
21.5	26.5	5	32%	5	100%					3 BROKEN FAULTED ROCK
26.5	31.5	5	70%	5	100%		21.5 - 31.5			-24.3F FLDX, STAINED FAULT ZONE AT 45° 3 large calcite veins at 60° 27.1F 68° 28.1F 70° FAULT FILLED
31.5	36.5	5	70%	5	100%					-30.0F 21 core FLDX 35° SWING 31.5F FLDX 20° 32.3F FLDX 15° 34.0F FLDX 10°
36.5	41.5	5	48%	5	100%		31.5 - 41.5			3 FAULTS II TO COKE - FAULTS AND SWING -38.2F 35° -40.0F 60° -42.5F 70° MED GRAY LIMESTONE
41.5	46.5	5	48%	5	100%					-57.0F FAULTS - FAULTS
46.5	51.5	5	58%	5	100%		41.5 - 51.5			-46.0F FLDX SWING II TO CORE
51.5	56.5	5	46%	5	100%		51.5 - 57.0			49.5F 50° FAULTS - FAULTS SWING 51.5 - 57.0F FAULTS 3 FAULTS AND FAULTS 52.0F II TO CORE FLDX SWING FAULTS, FAULTS
56.5	61.5	5	46%	5	100%		57-58.3			-57.0F 50° LIMESTONE, GRAY, FINE GRAINED, FAULT 58.3-66.5 LIMESTONE, GRAY, BLACK, FAULTS, FAULTS, FAULTS 60.0F II MED. GRAINED, GRAY, FAULTS, FAULTS 60.0F FAULT, FAULT, FAULT, FAULTS 62.5F 45°
61.5	66.5	5	46%	5	100%					66.5 - LIMESTONE, GRAY, BLACK, MED. FAULTS ORGANIC WITH FAULTS, FAULTS, FAULTS THICK FAULT, FAULT, FAULT, FAULTS, FAULTS
66.5	71	4.5	41.50	4.5	100%		66.5 - 71			68.5F 60°

Hole Size NQ 1.20 min, 4"Hole No. DH-06-4

## Geologist's Drilling Report



**Holcim**

Hole NO. DH-06-4

## GEOLOGIC LOG OF DRILL HOLE

Project Poverty Point 2000

### L.s. Thickness

### Top L.s. Elev.

Location Poverty Point, LA

### O.B. Thickness:

Bottom L.s. Elev.

Coordinates \_\_\_\_\_

Date Started 2-6-06

Total Depth of Hole 90.5

### **Collar Elevation**

Date Finished 2-7-06

Contractor b6a be DRILLING

Bearing and Dif. ✓

Hole Logged 2-7-06

Driller \_\_\_\_\_

Bearing and Dip \_\_\_\_\_

Water Level  No

Logged By: TDF-10001

CORE, RECOVERY

Water level 10.70

Logged By MCWMM

Hole Size NR R.R.D. min. 4"

Hole No. D17-06-4

# NORWEST Geotechnical - Rock Core Log - Fractures

Page 1 of 10

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER / RIG:

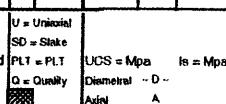
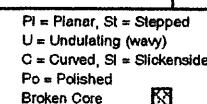
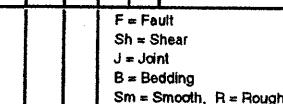
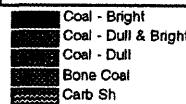
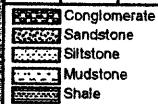
CLIENT: Holcim

DATE: 16 Feb 06

LOGGED BY: Eric Martin (ERM)

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	-	O-19.9 LS, v dk gray, fine to med grain. Varyably abt calcite veins 1-5mm. Zones of rubble w/ redrilled & caved material. Massive bedding	-	-	-	-	-	-	-
1	R 1	-	-	32	3	-	Closely Spaced	-	-	-
2	-	-	-	-	-	-	-	-	-	-
2.5	-	-	-	-	-	-	-	-	-	-
3	3.0	-	-	-	-	-	-	-	-	-
3	R 2	-	-	0	-	-	-	B	-	-
4	4	-	-	-	-	-	-	O	-	-
-	-	-	-	-	-	-	-	X	-	-
5.0	R 3	-	-	18	3	25	Rubble Lost Core	-	VCS	-
6	6	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-
7	R 4	-	-	52	3	32	J, Ir, R broke on vein Closely Spaced	1	CS	-
7.5	-	-	-	-	-	-	-	-	-	-
8	8	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-
9	R 5	-	-	0	3	52	J, Pl, S	-	CS	-
10	-	-	-	-	-	-	-	-	-	-
11	-	-	-	-	-	-	-	-	-	-
12	R 6	-	-	22	-	85	J, C, S, Ca vein	11.3	CS	-
13	-	-	-	-	-	-	-	-	-	-
13.5	-	-	-	-	-	-	-	-	-	-
14	R7	-	-	32	-	-	-	B O X	CS	-
15	-	-	-	-	-	-	-	2	CS	-



PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER/RIG:

CLIENT: HOLCIM

DATE: 16 Feb 06

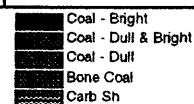
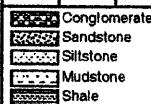
LOGGED BY: ERM

Run # From: To

Drilled: Rec: =

Core Dia: Type:

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	-	-	-	-	10	J, Pl, R, +r Cl	-	-	-
16	-	-	-	-	3	60	J, Pl, S, Ca vein	B	-	-
16.5	-	-	-	-	-	-	-	O	-	-
17	-	-	-	-	-	-	-	X	-	-
R	-	-	-	-	-	-	-	-	-	-
18	-	-	-	-	-	-	-	-	-	-
8	-	-	-	-	-	-	-	-	-	-
19	-	-	-	-	-	-	-	-	-	-
20	-	-	19.9-24.0 LS, med dk gray to dk gray, med grain to coarse grain. Massive, w occ high Fe Ca veins, bands of lt gray marble-like rock. Grad. cts'	76	3	10	J, Ir, R, Ca vein	2	-	-
21	21	-	-	-	-	90	J, C, R, +r Ca	21.0	-	-
22	-	-	-	-	3	-	-	B	-	-
R	-	-	-	-	-	-	-	O	-	-
23	-	-	-	79	-	-	-	X	-	-
9	-	-	-	-	-	-	-	-	10	CS
24	-	-	-	-	-	-	-	-	-	-
25	-	-	24.0-37 LS, pale ylw-brn/ylw gray, fine grain. Faint lam. Occ calcite veins	-	-	-	-	3	-	-
26	26	-	-	-	2	-	-	-	-	-
27	-	-	-	-	-	-	-	-	-	-
R	-	-	-	-	-	-	-	-	-	-
28	10	-	-	98	0	10	DI, C, R J, Pl, S,	-	1	WS
29	-	-	-	-	-	-	-	-	-	-
30	-	-	-	-	2	-	-	29.4	-	-

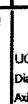


F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slicksided  
Po = Polished  
Broken Core

U = Uniaxial  
SD = State  
PLT = PLT

O = Quality



UCS = MPa  
Diametral = D -  
Axial = A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 3 of 10

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER / RIG:

DATE: 16 Feb 06

CLIENT: HOLCIM

LOGGED BY : ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	RUD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	-	-	-	-	-	-	-	-	-
31	31	-	-	-	-	-	-	-	-	-
32	R	-	-	-	-	50	J, Pl, S, thin Ca vein	B	-	-
33	11	-	-	88	2	-	-	0	-	-
34	-	-	-	-	-	-	-	X	-	-
35	-	-	-	-	-	-	-	-	-	-
36	36	-	-	-	-	-	-	-	-	-
37	R	37 - 53	LS, med dk gray to dk gray, fine to coarse grain. Variable unit. Mostly fg/dense w/ intervals cg w/ isolating pores, clayey zones	0	4	(5)	Becomes v. broken Clay, easy broken	4	-	-
38	12	-	-	-	-	-	-	-	-	-
39	39	-	-	-	-	-	-	-	-	-
40	R	40	-	-	3/4	-	-	40.2	-	-
41	13	-	-	35	-	-	-	B	-	-
42	-	-	-	-	-	-	-	0	-	-
43	43	-	-	-	-	-	-	X	-	-
44	R	44	-	-	3 1/4	-	-	-	-	-
45	14	-	-	-	-	-	-	-	-	-
							Clayey, crumbly	5	-	-
Conglomerate Sandstone Siltstone Mudstone Shale		Coal - Bright Coal - Dull & Bright Coal - Dull Bone Coal Carb Sh				F = Fault Sh = Shear J = Joint B = Bedding Sm = Smooth, R = Rough	Pl = Planar, St = Stepped U = Undulating (wavy) C = Curved, Sl = Slicksided Po = Polished Broken Core	U = Unload SD = Stake PLT = PLT Q = Quality	UCS = Mpa Diameter = D - Axial = A	

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER / RIG :

CLIENT: HOLCIM

DATE: 16 Feb 06

LOGGED BY : ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	HQD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	R							-		
46	14									
47	47									
48										
49	R									
50	15									
51										
52	52									
53	R									
54	16		53- 83 LS, dk gray, med to coarse grain. Mod abt high and low & calcite veins		8		Clayey/silty zones Wx/crumblly J, C, S	49.0		20+
55										
56										
57	52									
58	R									
59	17				11		disoln pores, weak Unit fails mostly on Ca Vein discord. DI, PI, R	6		8+
60										
						F = Fault Sh = Shear J = Joint B = Bedding Sm = Smooth, R = Rough	PI = Planar, St = Stepped U = Undulating (wavy) C = Curved, SI = Slicksided Po = Polished Broken Core	U = Unknown SD = Shale PLT = PLT Q = Quality	UCS = Mpa Diameter = D - Azial A	

PROJECT: HOLCIM - Poverty Point

LOCATION:

DRILLER/RIG:

CLIENT: HOLCIM -

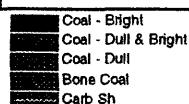
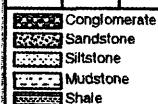
DATE: 17 Feb 06

LOGGED BY : ERM

HOLE No: DH-06-02

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	FOD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	R		LS, dk gray cont			90	J, C, R, Ca	60.6	-	
61	17					35	J, Pl, S, Ca	-	-	
-	R					-	-	B	-	
62	62					-	-	O	-	
-	R					-	-	X	-	
63						60	J, Pl, R, Ca	-	-	
64	18			95	3	60	J, Pl, R, Ca	7	-	
65						90	J, C, R, Ca	-	15+	CS
66						60	J, Pl, S, Ca	-	-	
67	67					-	-	-	-	
68						-	-	-	-	
69	R					-	-	-	-	
70	19			63	2	0	J, Pl, R, tr Cl	69.0	1	WS
71						-	-	B	-	
72	72					-	-	O	-	
73	R					-	-	X	-	
74	20					20	J, Pl, S, tr Cl	8	-	
75						-	-	-	-	



F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Broken Core

U = Uniaxial  
SD = State  
PLT = PLT  
Q = Quality

UCS = Mpa      Is = Mpa  
Diameter = D...  
Axial = A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 6 of 10

PROJECT: HOLCIIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER/RIG:

CLIENT: HOLCIIM

LOGGED BY: ERM

DATE: 17 Feb 06

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	RQD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing		
						Graphic & Dip TCA	Type & Description					
76			LS, dk gray, cont.		2	70	J, C, S, Ca					
77	77							77.4				
78	R			3								
79	21			49		90	J, Ir, R, Ca					
						60	J, Pl, S, Ca					
							J, Ir, R					
80						75	J, Pl, S, Ca					
81								9				
81.5												
82												
R	83		83-90, LS, dk gray w zones of ylwish-gray staining, med grain, massive. Mod broken, tr ylw-org clay on fx's. Grad ctc's	57								
22						3						
84						1/4						
85						28	J, Pl, R, + Cl					
							J, Pl, S					
86	86							86				
R	23											
87	87							B				
								O				
								X				
88	R											
24				15			J, Pl, R, + Cl					
89								10				
90												

Conglomerate

Sandstone

Siltstone

Mudstone

Shale

Coal - Bright

Coal - Dull &amp; Bright

Coal - Dull

Bone Coal

Carb Sh

F = Fault

Sh = Shear

J = Joint

B = Bedding

Sm = Smooth, R = Rough

Pl = Planer, St = Stepped

U = Undulating (wavy)

C = Curved, Sl = Slickensided

Po = Polished

Broken Core

U = Uniaxial

SD = Strike

PLT = PLT

O = Quality

D = D-

UCS = Mpa

Is = Mpa

Dinieral

Axial

A

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER / RIG :

CLIENT: HOLCIM

DATE: 17 Feb 06

LOGGED BY: ERM

Run #	From:	To:
Driiled:	Rec:	=
Core Dia:	Type:	

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	HOD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	R	90-94, LS, dk	gray, med to coarse grain, massive. Minor calcite veining, + r ylw-org clay on open fx's. Grad ctc's	15	2/3	-	J, Pl, S, Ca 3mm	B 0 X 10	-	-
91	24					60				
92	92					-				
93						60	J, Pl, S, + Cl	93.5	-	-
94	25			19		-			25+	VCS
95			94-110 LS, dk gray			-		B 0 X	-	-
96			w minor zones of ylw-org staining, med grain, massive. Minor calcite veining. Mod broken.			-				
97	97		Thin coatings of clay on most open fx surfaces, more so than above or below. Grad ctc's		3	60	J, Pl, S, Cl	11		
98	R			0		90	J, C, S, Cl		15+	CS
99	26					-				
100	100						Prominent, wavy vert fx's through most of interval	100.5		
101	R			0				B 0 X	10+	CS
102	27									
103	103							12		
104	R			18					20+	CS
105	28									

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planer, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Broken Core

U = Uniaxial  
SD = Strike  
PLT = PLI  
Q = Quality  
UCS = Mpa  
Diametral = D-  
Axial = A

**NORWEST**  
CORPORATION

**Geotechnical - Rock Core Log - Fractures**

Page 8 of 10

PROJECT: HOLCIM - Poverty Point

LOCATION:

DRILLER / RIG:

CLIENT: HOLCIM

DATE:

LOGGED BY : ERM

HOLE No: DH-06-02

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing			
						Graphic & Dip TCA	Type & Description						
106	R			18	3	45	J, Pl, S, Ca	B	-				
28						90	J, Pl, R, Cl	O	-				
107								X	-				
108	108							12	-				
109									-				
110	R								-				
29	29		110-116.5 LS, dk gray, med to coarse grain, massive. Mod calcite veining, grad ctc's	38	3	60	J, Pl, R, Ca	B	-				
111						0	J, Ir, R, Cl	O	-				
112								X	-				
113	113					3	J, Pl, R, Ca	13	-				
114						65	J, Pl, S, Ca		-				
115	R					40	J, Pl, S, Ca	20+	CS				
30	30		116.5 - 123.5 LS, dk gray, med grain, massive. Minor calcite veins, grad ctc's	26	3	90	J, Pl, S, Ca	25+	VCS				
116								117	-				
117									-				
118	118					3		B	-				
119	R					70	J, L, S, +r Cl	O	-				
31	31					20	J, Pl, S, Cl	X	-				
120								14	-				
									20+	CS			
						F = Fault Sh = Shear J = Joint B = Bedding Sm = Smooth, R = Rough		Pl = Planar, St = Stepped U = Undulating (wavy) C = Curved, Sl = Slickensided Po = Polished Broken Core		U = Uniaxial SD = State PLT = PLT Q = Quality		UCS = Mpa Is = Mpa Diametral = D - Asnl A	

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 9 of 10

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-02

LOCATION:

DRILLER / RIG :

CLIENT: HOLCIM

DATE: 17 Feb 06

LOGGED BY : FRM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (ft)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	HQD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing	
						Graphic & Dip TCA	Type & Description				
-	31					10	J, Pl, R	B			
21	121					45	J, C, S, Ca	O			
-	22	R						X			
-	23							14			
-	32					33	J, Pl, S				
-	24					65	J, Pl, S	124			
125		123.5 - 138 LS, 1+ brn-gray to med dk gray						B			
125.5								O			
26								X			
-	27	R				55	J, Pl, S	15			
-	33					90	J, C, S				
-	28					39					
-	29					3					
130		130.5				90	J, C, S, Ca	133.5			
31								B			
32		R						O			
33								X			
34								16			
33	34										
135	135										

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slicksided  
Po = Polished  
Broken Core

U = Unknown  
SD = Shear  
PLT = PLT  
O = Quality

UCS = Mpa  
Is = Mpa  
Diemtral - D -  
Axial - A

**NORWEST**  
CORPORATION

## **Geotechnical - Rock Core Log - Fractures**

Page 10 of 10

**PROJECT: HOLCIM - Poverty Point**

**HOLE No:** DH - 06 - 02

**LOCATION :**

DRILLER/RIG

DATE: 17 Feb 06

**CLIENT:** HOLCIM

LOGGED BY : ERAN

Run #      From:      to

Run #      From:      to

Drilled: Rec: =

Core Dia: Type:

**NORWEST** CORPORATION Geotechnical - Rock Core Log - Fractures

Page 1 of 6

**PROJECT: HOLCIM - Poverty Point**

HOLE No: AH-06-03

**LOCATION:**

Logged at Devil's Slide Quarry Shop

DEBUT / FB / BIG

DATE: 15 Feb 06

CLIENT: Holcim

LOGGED BY : Eric Martin

RUN (#)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Sample	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dig TCA	Type & Description			
-	-	-	-	0	3 1/4	-XX X XX X -X XX X XX X XX X -	Rubble 8 Lost Core 1.5	0	-	-
-	1	-8	-	-	-	-	-	-	-	VCS
-	1.5	X	1.5 Lost Core	-	-	-	-	-	-	-
-	2.5	-	0-10 med-fine gr LS, v dk gray	0	3 1/4	-XX X XX X XX X XX X -	Rubble w a few sm intact pieces	-	-	-
-	2	-	-	-	-	-	3.5	-	-	VCS
-	3.5	X	-	-	-	-	-	-	-	-
-	R	-	4.0-6.0 High & calcite filled healed fx's 5-10 mm	0	3 1/4	-XX X XX X -	-	B	-	-
-	3	-	-	-	-	-	-	0	-	-
-	5.0	-	-	-	-	-	-	X	-	-
-	6	-	-	-	-	-	-	-	-	-
-	Run	-	-	0	3 1/4	-YX X -XX Y XX X XX X XX X XX X -	Rubble w a few intact pieces	-	-	-
-	4	-	-	-	-	-	-	1	-	VCS
-	7.5	-	-	-	-	-	-	-	-	-
-	8.0	-	-	-	-	35	B 25° Pl, R, Clean	-	-	-
-	Run	-	-	0	3 1/4	-	8.60	-	-	-
-	5	-	-	-	-	-	Rubble	-	-	VCS
-	10	X	Lost Core	-	-	-X	1.7	10.0	-	-
-	10.0	-	10-12.4 occ, mostly thin high & Ca veins	20	-	-	10.65 J 20° Pl, R, Ca	-	-	-
-	10	-	-	20	-	-	11.33 J 70° Pl, R, Ca	B	-	-
-	11	-	-	20	-	-	11.43 J 70° Pl, R, Ca	0	-	-
-	11	-	10-15.4 med gr. LS, med gray	30	-	-	11.45 J 20° Pl, R, Ca	X	-	-
-	11	-	-	75	-	-	11.70 Sh 30° Pl, R, .07 Ca/Cl f ill.	-	11	CS
-	6	-	-	91	-	-	12.2 J 75° Pl, R, Ca	2	-	-
-	14	-	14-15 A few high & Calcite veins	30	-	-	14.60 J 30°, Pl, R, +rCa	-	-	-
-	15.0	-	-	60	-	-	15.00 J 80°, Pl, R, Ca	-	-	-

**Conglomerate**  
**Sandstone**  
**Siltstone**  
**Mudstone**  
**Shale**

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carr. Sh.

**F = Fault**  
**Sh = Shear**  
**J = Joint**  
**B = Bedding**  
**Sm = Smooth, R = Rough**

**PI = Planar, SI = Stepped**  
**U = Undulating (wavy)**  
**C = Curved, SL = Slickensides**  
**Po = Polished**  
**Benton Corp.**


U = Uniform	
SD = State	
PLT = PLT	UCS = Mpa      Is = Mpa
Q = Quality	Diameter = D

**NORWEST** CORPORATION Geotechnical - Rock Core Log - Fractures

Page 2 of 6

**PROJECT:** Holcim - Poverty Point

HOLE No: PH - 06 - 03

**LOCATION:**

DBII | EFB / BIG

CLIENT: Helms

DATE: 15 Feb 06

Run # From: to

Drilled: Rec:

Cost Plus

RUN (#)	DRILL DEPTH	Organic Log	LITHOLOGIC DESCRIPTION	ROD %	SINTERED	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TGA	Type & Description			
-	-	-	15.4 - 17.4 fine gr. LS., dk gray	-	-	25	15.46 J 35° PI, R, Ca	-	-	-
16	-	-		-	3	30	16.2 J 30°, PI, R, Ca	B	-	-
17	R	u		97	-	30	16.45 J 30°, PI, R, Ca	D	-	-
18	u	n		-	-	-	-	X	-	-
19			17.4 - 22.7, LS, med grain, med gray	-	-	50	Ca vein 2-5 mm	2	-	-
20	20.0	7		-	-	50	18.60 J 50°, S, R, Ca	-	3	MWS
21	R	u		-	-	30	19.80 J 30°, P, R, Cl Fe	19.2	-	-
22	u	n		-	-	-	-	-	-	-
23	8			88	-	-	20.7 B 15°, I, R, C	-	-	-
24			22.7 - 29.8 LS, fine grain, dk gray, fossils at 26'	3	-	-	21.45 J 75°, P, S, Ca Fe	-	-	-
25	25.0			-	-	-	21.5 J 10°, I, R, Ca Cl	-	-	-
26	R	u		-	-	3	22.50 Ind 0°, Clean	B	-	-
27	u	n		-	-	-	23.08 J 10°, I, R, Ca Cl	D	-	-
28	9			94	-	-	24.55 J 50°, P, S, +tr Ca	X	-	-
29				-	-	-	24.85 J 20°, I, R, +tr Cl	3	-	-
30				-	-	-	-	-	-	-
				-	-	-	26.33 B 30°, C, S, C	-	-	-
				-	-	-	26.68 B 40°, I, R, C	-	-	-
				-	-	-	27.37 B 30°, PI, R, C	-	-	-
				-	-	-	28.10 DI 0°, Clean	6	MWS	-
				-	-	-	28.28 J 15°, I, R, +tr Cl	-	-	-
				-	-	-	29.4 DI 50°, I, R, C	-	-	-
				-	-	-	29.74 J 15°, I, R, Ca Cl	-	-	-
				-	-	-	29.9 J 10°, PI, R, +tr Cl	4	-	-

DI = Drill Induced I = Indeterminate

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 3 of 6

PROJECT: Holcim Poverty Point

HOLE No: DH-06-03

LOCATION:

DRILLER/RIG:

DATE: 15 Feb 06

CLIENT: Holcim

LOGGED BY: ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing	
						Graphic & Dip TCA	Type & Description				
31	R u n		29.8 - 43.2 LS, med to fine grain, med dk gray to dk gray				31.15 J 40°, S, R, Ca	B	-		
32	10		calcite vein 10-15mm	87	3		32.55 J 10°, C, R, C	0	-	5	MWS
33							32.70 J 10°, PI, R, C	X	-		
34			calcite vein 15mm				33.1 J 75°, PI, R, Ca 10-15mm Partly leached open	4	-		
35	R u n						33.9 J 35°, I, R, C				
36							34.60 J 60°, PI, R, leach				
37							35.0 J 20°, I, R, Ca	37.0	-	12	LS
38	11				74		35.4 J 60°, PI, R, Ca				
39							35.5 J 30°, C, R, FeOx				
40							35.9 J 15°, PI, R, Ca				
41	R u n						36.5 J 50°, PI, R, Ca				
42							37.0 J 45°, PI, S, Ca				
43	12		43.2 - 46.0 Several prominent high Z joints w strong leaching + open cavities	81	3		38.0 J 40°, PI, S, Ca	B	-		
44							38.4 J 30°, St, S, Ca	0	-		
45							39.3 J 75°, PI, S, Ca	X	-		
							39.4 J 35°, PI, S, Ca				
							39.6 I 10°, PI, R, C				
							40.0 DI 0°				
							40.55 DI 20°, I, R, C	5	-		
							41.32 J 05°, C, S, Ca				
							42.37 DI 15°, C, S, C				
							43.25 J 60°, C, R, Ca			4	MWS
							44.0 DI, I, R, C				
							44.5 - 46.0 J 75°, C, R, leached w vugs				
							45.0 DE				

Conglomerate

Sandstone

Siltstone

Mudstone

Shale

Coal - Bright

Coal - Dull &amp; Bright

Coal - Dull

Bone Coal

Carb Sh

F = Fault

Sh = Shear

J = Joint

B = Bedding

Sm = Smooth, R = Rough

PI = Planar, St = Stepped

U = Undulating (wavy)

C = Curved, Si = Slickensided

Po = Polished

Broken Core

U = Unknown

SD = Slick

PLI = PLI

D = Dull

Asci = A

UCS = Mpa

Is = Mpa

Diametral = D ..

Asci = A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 4 of 6

PROJECT: Holcim - Poverty Point

HOLE No: OH-06-03

LOCATION:

DRILLER/RIG:

CLIENT: Holcim

DATE: 15 Feb 06

LOGGED BY: ERM

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (#)	DRILL DEPTH	Geoph Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Orient & Dip TCA	Type & Description			
-	-	-	-	-	-	-	J cut from above	46	-	-
46	R	-	46 - 53.4 LS, med grain, med dk grey, abt mostly high & calcite veins 1-10mm	3	-	-	-	-	-	-
42	U	-	-	68	-	-	47.5 J 75°, Pl, S, C closed below 47.5	B	-	-
48	U	-	-	-	-	-	47.5 J 20° I, R, +r Cl	B	-	-
49	-	-	-	-	-	-	48.4 J 50°, L, S, C	0	-	-
50	-	-	-	-	-	-	49.8 DI	6	-	-
51	R	-	-	89	3	-	50.0 DI	-	-	-
52	U	-	-	-	-	-	-	-	-	-
53	U	-	-	-	-	-	51.2 J 10°, C, R, Ca Fe	-	-	-
54	-	-	-	-	-	-	-	-	-	-
55	-	-	-	-	-	-	52.4 J 50°, P, R, Ca	6	MWS	-
56	R	-	53.4 - 65.8 LS, med to fine grain, med dk grey. Occ fine calcite veins < 1mm	3	-	-	52.9 J 05°, I, R, +r Cl	-	-	-
57	U	-	-	-	-	-	54.38 J 05°, I, R, C	-	-	-
58	U	-	-	-	-	-	54.95 J 20°, Pl, S, +r Cl	55	-	-
59	-	-	-	-	-	-	55.30 J 60°, C, R, Ca	-	-	-
60	-	-	-	77	3	-	56.6 J 05°, P, S, C	B	-	-
59	R	-	-	-	-	-	-	0	-	-
61	U	-	-	-	-	-	57.53 J 60°, I, R, +r Ca	7	-	-
62	U	-	-	-	-	-	57.9 J 90°, C, R, Ca	-	-	-
63	-	-	-	-	-	-	58.45 J 40°, I, R, C	-	-	-
64	-	-	-	-	-	-	59.35 J 70°, Pl, S, Ca	-	-	-
65	-	-	-	-	-	-	59.7 J 65°, Pl, S, Ca	-	-	-

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Bro = Broken Core

U = Univisi  
SD = Slati  
PLT = PLT  
Q = Quatly

UCS = Mpa      K = Mpa  
Diameter = D...  
Aspect = A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 5 of 6

PROJECT: Holcim - Poverty Point

HOLE No: DA-06-03

LOCATION:

DRILLER / RIG:

CLIENT: Holcim

DATE: 15 Feb 06

LOGGED BY: ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	R					60.1 J 30°, VI, R, C1 Ca	-	-	
61	Un						60.45 J 45°, C, S, C	B	-	
62	16			68	3		61.45 DI	B	-	
63							62.35 DI	X	-	
64							62.4 - 63.3 J 80°, PI, S, Ca	7	-	
65							63.55 J 20°, C, S, C		8	CS
66	R		65.8 - 74 LS, med + fine grain, med-kdry. Occ calcite veins 1-5 mm				64.45 DI			
67	Un						64.75 DI			
68							65.05 DI			
69							65.65 J 20°, I, R, +r C1			
70										
71	R							B	-	
72	Un							O	-	
73								X	-	
74									4	MWS
75										
76										
77	18			77	3		68.1 J 70°, PI, S, Ca			
78							68.9 J 15°, I, R, Ca	8		
79							69.95 DI			
80	R						70.05 DI			
81	Un						70.74 DI			
82										
83										
84										
85										
86										
87										
88										
89										
90										
91										
92										
93										
94	19			94	3		72.1 J 30°, I, R, Ca		6	MWS
95							73.05 J 20°, PI, R, +r C1			
96							73.15 J 25°, PI, R, +r C1			
97							73.15 - 73.6, J 90°, L, S, Ca	73.5		
98							73.7 J 70°, PI, R, Ca			
99							74.5 J 50°, PI, S, C			
100							75.0 J 20°, C, R, +r C1			

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Bro = Broken Core

U = Uniaxial  
SD = Strike  
PLT = PLT  
Q = Quality

UCS = Mpa  
Diameter = D ..  
Awi = A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 6 of 6

PROJECT: Holcim - Poverty Point

HOLE No:

LOCATION:

DRILLER / RIG:

DATE: 15 Feb 06

CLIENT: Holcim

LOGGED BY: ERM

Run # From: To

Driiled: Rec. =

Core Dia. Type:

RUN (#)	DRILL DEPTH	Grain Len	LITHOLOGIC DESCRIPTION	RQD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	-	74-80.7 LS, olive or dk brownish gray, fine grain	-	-	-	75.09 J25°, PI, R, +rc Cl	-	-	-
76	R	2		-	-	-	76.3 J40°, C, R, +rc Cl	-	-	-
-	n	-		-	-	-	76.9 J60°, C, S, Ca	B	-	-
77	19			96	-	-	77.27 J20°, PI, S, +rc Cl	O	-	-
-	-	-		-	-	3	78.65 J60°, PI, S, Ca	X	-	-
28	-	-		-	-	-	79.8 J55°, PI, S, C	-	7	MWS
29	-	-		-	-	-	-	-	-	-
30	-	-		-	-	-	-	-	-	-
80	-	-	80.7- 84.3 LS, med grain, med dk gray. Mod abt calcite veins <1 mm. Locally very broken along veins.	80.7	3	-	80.45 J60°, PI, S, C 80.65 J60°, I, R, +rc Cl 80.75-82.1 J90°, I, R, Ca fx's along Ca veins.	-	-	-
81	R	un		-	-	-	82.1 J35°, PI, R, Ca 82.45 J50°, PI, S, Ca	-	-	-
-	n	-		-	-	-	-	-	-	-
82	20			55	3/4	-	-	82.1	-	-
-	-	-		-	-	-	-	-	-	-
83	-	-		-	-	-	83.1 J20°, I, R, Ca	-	-	-
84	-	-		-	-	-	-	-	-	-
84.6	-	-	Lost Core	84.3	-	-	84.35 J, I, R, Cl Lost	B	-	-
85	-	-		-	-	-	-	O	-	-
85	-	-		-	-	-	-	X	-	-
-	-	-		-	-	-	-	-	-	-
86	R	un	84.3-90 LS, fine grain, v dk gray	-	-	-	85-85.75 J90°, I, R, Ca	10	-	-
-	n	-		-	-	-	-	-	-	-
87	-	-		-	-	-	86.05 J30°, L, S, Ca	-	-	-
87	-	-		-	-	-	86.15-86.23 Gauge zone filled w/ clay & rock frags	-	-	-
88	-	-		-	-	-	86.45 J20°, PI, S, Cl	-	-	-
89	-	-		-	-	-	86.6 J25°, PI, S, Cl	-	-	-
90	-	-		-	-	-	87.45 J40°, PI, S, Ca Cl	-	-	-
91	-	-		-	-	-	88.0 J25°, PI, R, +rc Cl	-	-	-
92	-	-		-	-	-	88.07 J30°, PI, R, Cl	-	-	-
93	-	-		-	-	-	-	-	-	-
94	-	-		-	-	-	89.05 DI	-	-	-
95	-	-		-	-	-	89.6 J55°, PI, S, Ca	-	-	-
96	-	-		-	-	-	89.95 J45°, C, R, Ca	-	-	-
TD = 90 Feet	-	-		-	-	-	-	-	-	-
Conglomerate	Coal - Bright	F = Fault	PI = Planar, St = Stepped	U = Uniaxial						
Sandstone	Coal - Dull & Bright	Sh = Shear	U = Undulating (wavy)	SD = Shale						
Siltstone	Coal - Dull	J = Joint	C = Curved, Sl = Slicksided	PLT = PLT						
Mudstone	Bone Coal	B = Bedding	Po = Polished	O = Quality						
Shale	Carb Sh	Sm = Smooth, R = Rough	Broken Core	Axial						
				A						

**NORWEST** Geotechnical - Rock Core Log - Fractures

Page 1 of 6

PROJECT: HOLCIM - Poverty Point

HOLE No: OH-06-04

LOCATION:

DRILLER / RIG:

CLIENT: HOLCIM

DATE: 15 Feb 06

LOGGED BY: Eric Martin (ERIN)

Run #	From	To
Drilled	Rec	=
Core Dia.	Type	

RUN (#)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Stamp	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Orient & Dip TCA	Type & Description			
-	R 1		0-2 LS, med dk gray, coarse grained, abt calcite veins	0	-	-	-	-	-	4 CS
-	1.5				3	-	-	-	-	
-	2					-	-	-	-	
2.5						-	-	-	-	
3	R 2		2-9 LS, dk gray, med gray. Massive to faint lams. Mod calcite veins	60	15	J, PI, R, CI		B 0 X	-	7 CS
4	2				60	J, C, R, isoln of calcite		1	-	
5						-	-	-	-	
6						-	-	-	-	
7	R 3				57	J, PI, R, lt brn silt/clay		-	-	12 CS
7.5						-	-	-	-	
8						-	-	-	-	
9						-	-	-	-	
10	R 4		10-11.5 LS, lt olive gray and v dk gray, fine grain, massive. Clay on fr's, zones of rubble	22	90	J, C, R, CI		16.5	-	5+ CS
11	4					-	-	-	-	
11.5						-	-	-	-	
12						-	-	-	-	
13	R 5				3	minor isoln pits		B 0 X	-	
14	5					-	-	2	-	14 CS
15			11.5- 13.4 LS, dk gray, med to coarse grained, see below.	60	90	J, C, S, Ca		-	-	
					10	J, PI, R, +r CI		-	-	

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

PI = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, SI = Slickensided  
Po = Polished  
Broken Core

U = Unplanar  
SD = Slaty  
PLT = PLT  
Q = Quality  
Diametral = D...  
Axial = A

# NORWEST Geotechnical - Rock Core Log - Fractures

Page 2 of 6

PROJECT: HOLCIM

LOCATION:

DRILLER / RIG:

CLIENT:

DATE: 17 Feb 06

LOGGED BY: ERM

HOLE No: DH - 06-04

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	Coref. Log	LITHOLOGIC DESCRIPTION	FAD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Oriented & Dip TCA	Type & Description			
-	-	-	-	-	60	-	J, Pl, S	-	-	-
16	16.5	-	11.5-53.4 LS, dk gray to med dk gray, med grain, locally cg.	-	-	-	-	B	-	-
-	-	-	Massive w occ faint streaks/lams. Minor fine calcite veins w occ thick veins 2-15mm	51	2	10	J, Pl, R, C1	0	-	-
18	R	6	-	-	-	-	-	X	-	-
-	-	-	-	-	-	-	-	-	-	-
19	-	-	-	-	-	-	-	-	-	-
20	-	-	-	-	-	-	-	2	-	-
-	-	-	-	-	-	-	-	-	-	-
21	-	-	-	-	-	-	-	-	-	-
21.5	-	-	-	-	30	-	J, Pl, S	20.3	-	-
22	-	-	-	-	-	-	-	B	-	-
-	-	-	-	-	-	-	-	0	-	-
23	R	-	-	-	2	-	-	X	-	-
-	-	-	-	-	-	-	-	-	-	-
24	-	7	-	35	-	-	-	3	-	-
-	-	-	-	-	-	-	-	-	-	-
25	-	-	-	-	90	-	J, C, R, Ca vein w disoln	-	-	-
-	-	-	-	-	-	-	-	-	-	-
26	-	26.5	-	-	-	70	/ J, Pl, R, Ca w disoln	-	-	-
-	-	-	-	-	-	-	-	-	-	-
27	-	-	-	-	90	-	J, C, R, Ca	29.4	-	-
-	-	-	-	-	-	-	-	-	-	-
28	R	-	-	83	2	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-
29	-	8	-	-	-	-	-	8	-	-
-	-	-	-	-	-	-	-	-	-	-
30	-	-	-	-	-	-	-	-	-	-

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh.

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Broken Core

U = Unlaminated  
SD = Shale  
PLT = PLT  
D = Clayey

UCS = Mpa  
Is = Mpa  
Diameter = D -  
A = A

# NORWEST Geotechnical - Rock Core Log - Fractures

Page 3 of 6

PROJECT: HOLCIM - Poverty Point

HOLE No: OH-06-04

LOCATION:

DRILLER/RIG:

DATE: 17 Feb 06

CLIENT: HOLCIM

LOGGED BY: ERM

Run #	From:	To:
Drilled:	Rec:	=
Core Dia:	Type:	

RUN (#)	DRILL DEPTH	Graphic Log	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Orient & Dip TCA	Type & Description			
-	-	-	11.5 - 53.4 LS, dk gray cont.	83	-	10	J, I, R, Cl	-	-	-
31	31.5	R		-	-	10	J, I, R, Cl	B	-	-
32	9			-	-	60	J, C, R, Ca	0	-	-
34	-			-	-	20	J, C, R, Cl	X	-	-
35	-			76	-	-	-	4	-	-
36	36.5	R		-	-	90	J, I, R, Ca	10	CS	-
37	10			-	-	-	Ireg vert fx zone	-	-	-
38	-			-	-	-	Along dolom calcite veins. Very broken	37.9	-	-
39	-			39	-	-	-	-	-	-
40	-			-	-	-	-	-	-	-
41	41.5	R		-	-	60	J, Pl, S, Ca	10	CS	-
42	11			-	-	-	-	-	-	-
43	-			79	-	-	-	-	-	-
44	-			-	-	10	J, C, R, +r Cl	7	CS	-
45	-			-	-	-	-	-	-	-

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

Pl = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, Sl = Slickensided  
Po = Polished  
Broken Core

U = Uniaxial  
SD = State  
PLT = PLT  
G = Quality

UCS = Mpa  
D = Mpa  
Axial A

# NORWEST Geotechnical - Rock Core Log - Fractures

Page 4 of 6

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-04

LOCATION:

DRILLER/RIG:

CLIENT: HOLCIM

DATE: 17 Feb 06

LOGGED BY: ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	Dip/Top Log	LITHOLOGIC DESCRIPTION	RHO kg/m³	Skirted	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Dip & Dip TCA	Type & Description			
46	46.5		11.5-53.4 LS, dk gray cont.	79				46.5		
47						75	J, PI, S			
48	R							8		
49	12			64	2			0		
50						90	J, I, R, 1-2 mm Ca vein +rc cl	6		
51	51.5									
52										
53	R		53.4-57 LS, med gray, coarse grain, massive. Disoln pits, minor calcite veins, Grad etc's	41		90	J, C, S J, I, R, +rc cl J, PI, S			
54	13									
55						3		55.4		
56	56.5									
57			57-58.8 LS, dk gray, fine grain			90	J, C, S, +rc cl			
58	R		Chert bands 10-20cm Faint lams, grad etc's				J, I, R on chert	7		
59	14			54	2					
60						10	J, PI, R, C1			

Conglomerate  
Sandstone  
Siltstone  
Mudstone  
Shale

Coal - Bright  
Coal - Dull & Bright  
Coal - Dull  
Bone Coal  
Carb Sh

F = Fault  
Sh = Shear  
J = Joint  
B = Bedding  
Sm = Smooth, R = Rough

PI = Planar, St = Stepped  
U = Undulating (wavy)  
C = Curved, SI = Slickensided  
Po = Polished  
Broken Core

U = Unusual  
SD = Stake  
PLT = PLT  
Q = Quirky

UCS = Mpa  
Direct = U..  
Axial A

NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 5 of 6

PROJECT: HOLCIM - Poverty Point

HOLE No: DH - 06 - 04

LOCATION:

DRILLER/RIG:

CLIENT: HOLCIM

DATE: 17 Feb 06

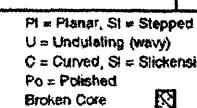
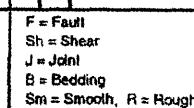
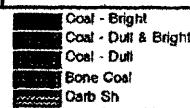
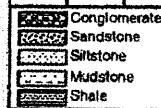
LOGGED BY: ERM

Run # From: to

Drilled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	Graphic & Dip TCA	LITHOLOGIC DESCRIPTION	ROD %	Strength	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
61	61.5		58.8 - 69.3 LS, dk gray, med to Coarse grain, lam	54				B 0 X		
62	R							7		
63	15									
64			64-66.5 Abt calcite veins 2-15mm	51		85	J, Pl, S, ca	64.5	11	CS
65										
66	66.5					45	J, Pl, S	B 0 X		
67						2		8		
68	R					70	J, Pl, S			
69	16		69.3 - 81.4 LS, med dk gray to dk gray, fine grained, faint lam.	78		32	J, Pl, S, +r Cl		11	CS
70			Bands / Nodules blk Chert 15-30 mm							
71	71									
72	R									
73	17			98				73.6	5	MWS
74						25	J, Pl, R			
75							J, Pl, R, +r Cl/Fe Ox			



NORWEST  
CORPORATION

## Geotechnical - Rock Core Log - Fractures

Page 6 of 6

PROJECT: HOLCIM - Poverty Point

HOLE No: DH-06-04

LOCATION:

DRILLER / RIG:

DATE: 17 Feb 06

CLIENT: HOLCIM

LOGGED BY: ERM

Run # From: to

Driiled: Rec: =

Core Dia: Type:

RUN (#)	DRILL DEPTH	GRANITE LOG	LITHOLOGIC DESCRIPTION	RCG %	Stamps	DISCONTINUITIES		Core Box	SAMPLES	Fracture Spacing
						Graphic & Dip TCA	Type & Description			
-	-	-	69.3-81.4 LS cont. med dk gray w chert	-	-	60	J, Pl, S	-	-	-
26	76	R		-	-	40	J, Pl, R, Ca, on chert	B	-	-
77	81	X		81	-	-		O	-	-
78	81			-	-	-		X	-	-
79	81			-	-	-			-	-
80	81			-	-	-		9	-	-
81	81			-	-	-			-	-
82	82	R	81.4-88.6 LS, dk gray, med to coarse grained, faint lams	95	2	70	J, Pl, R	82.4	-	-
83	83		to massive Sharp ctc at base	95	-	30	J, Pl, S	B	-	-
84	84			-	-	-		O	-	-
85	85			-	-	-		X	-	-
86	86	R		-	-	-		10	-	-
87	87	20		69	-	10	J, Pl, R, +rcI		-	-
88	88		88.6-90.5 LS, lt olive gray, fine, grained, massive	-	-	-			-	-
89	89			-	-	90	J, C, S, Ca		-	-
90	90	R		0	-	-			-	-
.5	21				-	-			-	-
			TD 90.5 ft							
Conglomerate			Coal - Bright		F = Fault	Pl = Planar, St = Stepped	U = Unique			
Sandstone			Coal - Dull & Bright		Sh = Shear	U = Undulating (wavy)	SD = Shale			
Siltstone			Coal - Dull		J = Joint	C = Curved, SI = Slickensided	PLT = PLT			
Mudstone			Bone Coal		B = Bedding	Po = Polished	O = Quality			
Shale			Carb Sh		Sm = Smooth, R = Rough	Broken Core	UCS = Mpa	I = Mpa		
							Diamond	D ..		
							Axial	A		

**APPENDIX B**

**Core Photographs**

NO. 7978

91

1-800-4

Bo Lo

HOLLIM - Poverty Point  
Drill Hole - 06-02

Bal 0-11.3



NO. 79

JB

188-1



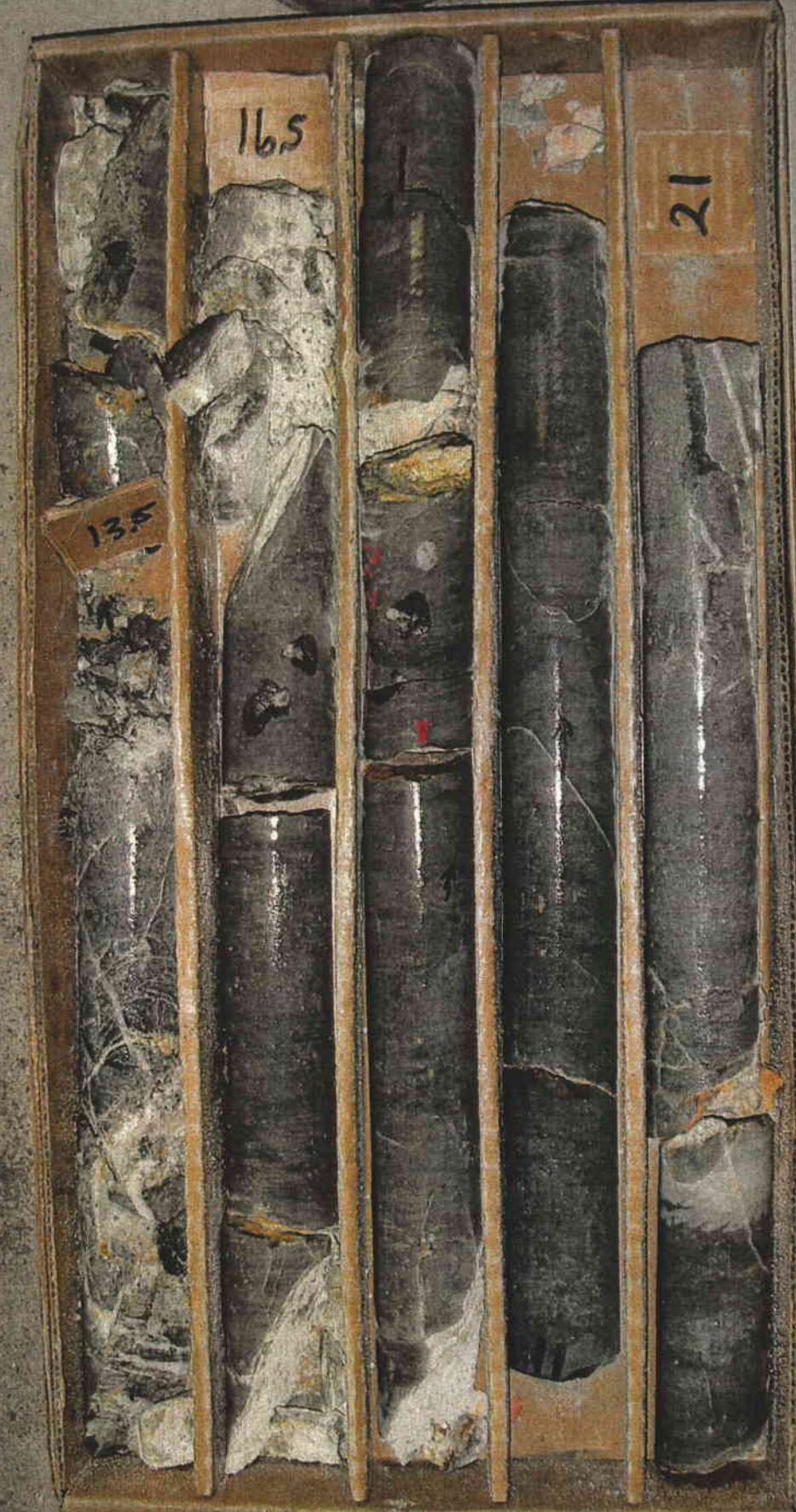
HOLLIM - Poverty Point  
Drill Hole - 06-02

Box 2 11.3-21

135

165

21



**EAR**

**TOP**

HOLLIM - Poverty Point  
Drill Hole - 06-02

Bm 3 21 - 294



YEAR

40

Q TOP

HOLCOM - Poverty Point  
Drill Hole - 06-02

Box# 29.4-40.2

36



YEAR  
740  
HQ TO

HOLLIM - Poverty Point  
Drill Hole - 06-02  
Box 40.2. 49



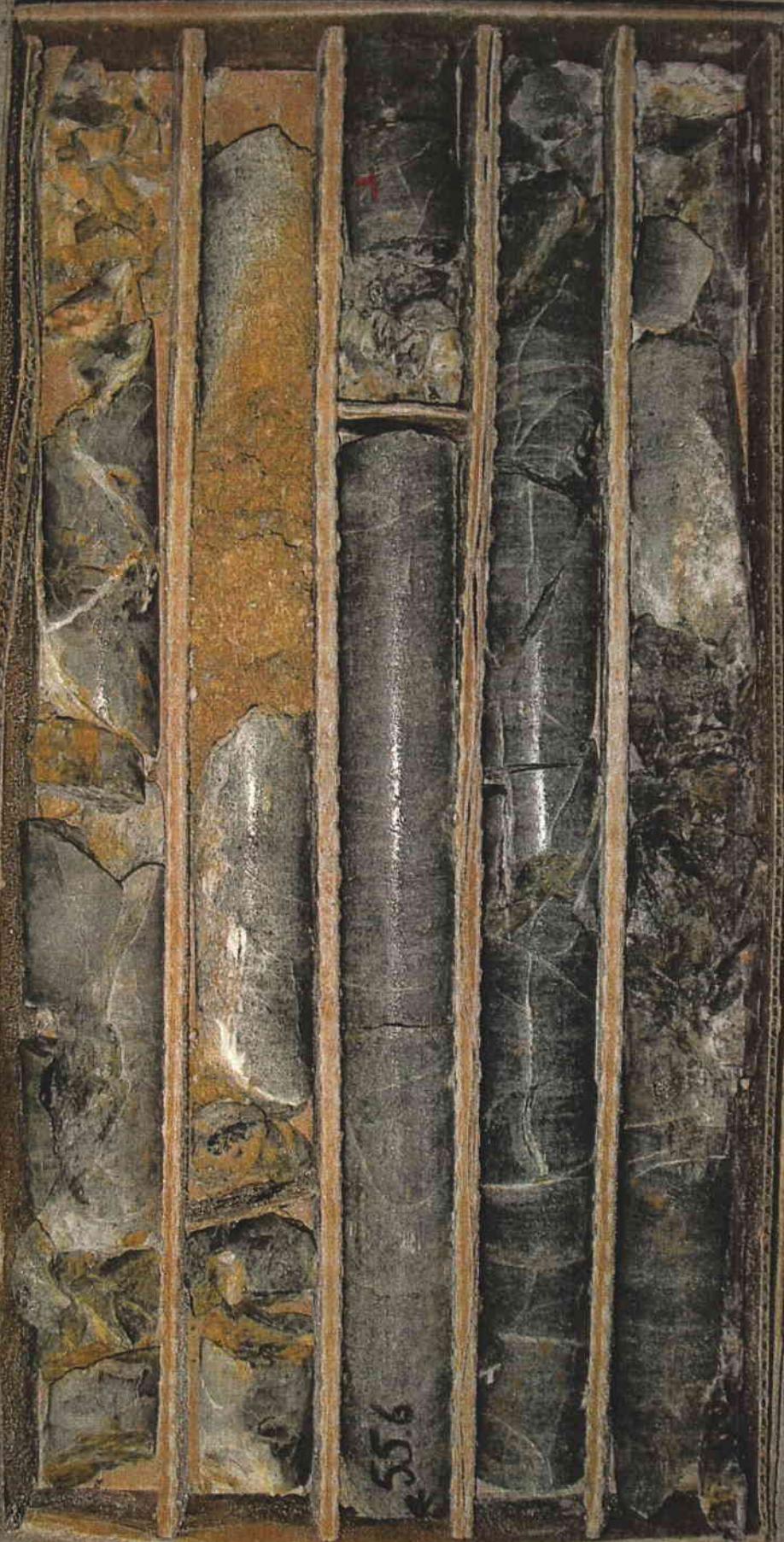
YEAR  
740

IQ TOP

HOLLIM - Poverty Point  
Drill Hole - 06-02

Box 6 49-60.6

556



YEAR

40

Q TO

HOLLIM - Poverty Point  
Drill Hole - 06-02  
Box # 60.6 - 69

b2



HOLLUM - Poverty Point  
Drill Hole - 06-02  
Box 8 69-774

PYEAR  
8740

NQ TO

HOLLIM - Poverty Point  
Drill Hole - 06-02

Aug 9 77.4. 86



92

HOLLIM - Poverty Point  
Drill Hole - 06-02

Box 10 86 - 93.5

HOLLIM - Poverty Point  
Drill Hole - 06-02

Bore 93.5-100.5



YEAR  
TO

HOLLIM - Poverty Point  
Drill Hole - 06-02

Box 12 100.5. 108



HOLLM - Poverty Point  
Drill Hole - 06-02

Box 13 108-117



HOLIM - Poverty Point  
Drill Hole - 06-02

Box 14 117-124



**YEAR**  
19  
HOLLM - Poverty Point  
Drill Hole - 06-02

Box 15 124-138.5



YEAR

10

TOP

HOLCOM - Poverty Point  
Drill Hole - 06-02

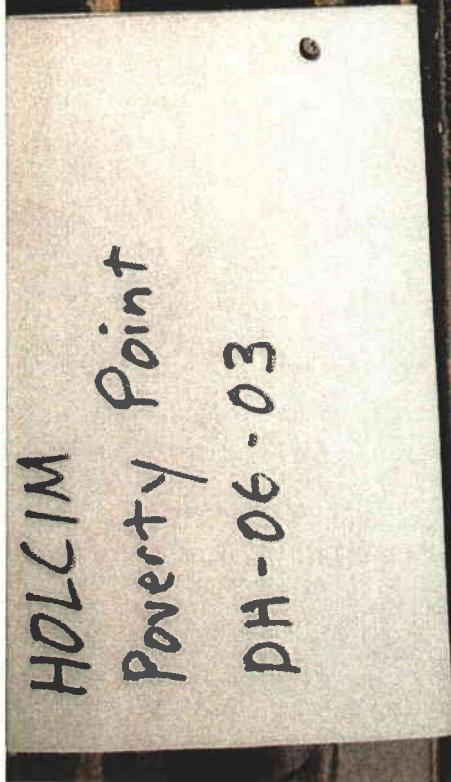
Box 16 138.5-198



HOLCIM  
Poverty Point  
DH - 06 - 03

D TOP

HOLCIM  
Poverty Point  
DH - 06 - 03



HOLCIM  
Poverty Point  
PH - 06 - 03

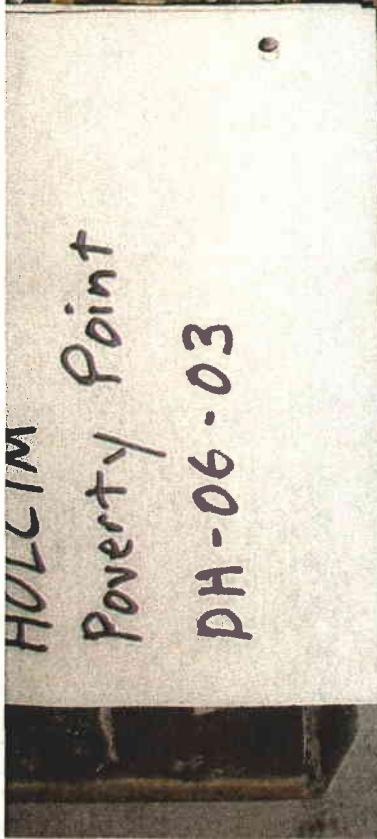
HOLCIM  
Poverty Point  
DH - 06 - 03

HOLCIM  
Poverty Point  
DH - 06 - 03

HULLIM

Poverty Point

DH - 06 - 03

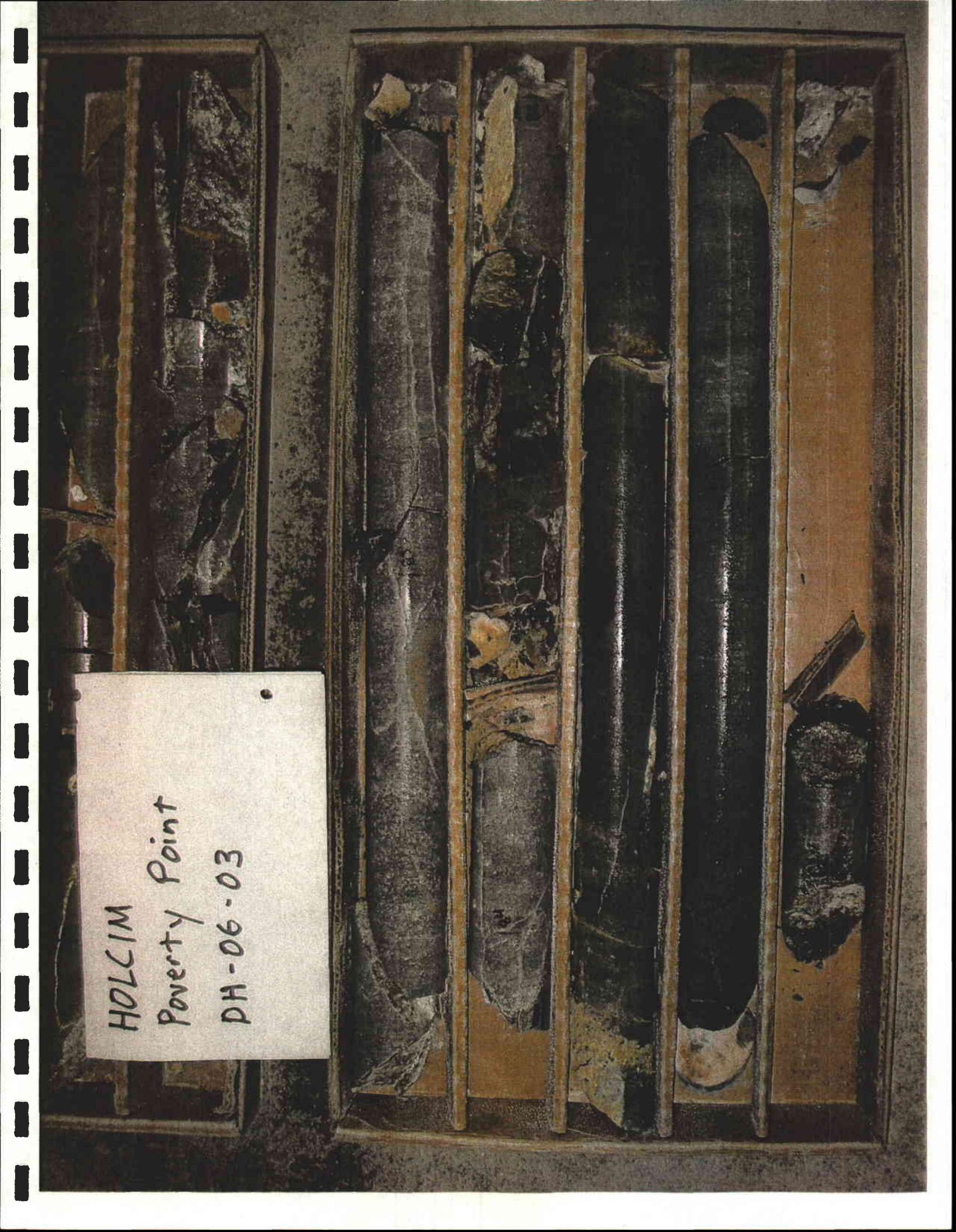


HOLCIM  
Poverty Point  
DH - 06 - 03

HOLCIM  
Poverty Point  
DH - 06 - 03

HOLCIM  
Poverty Point  
DH - 06 - 03





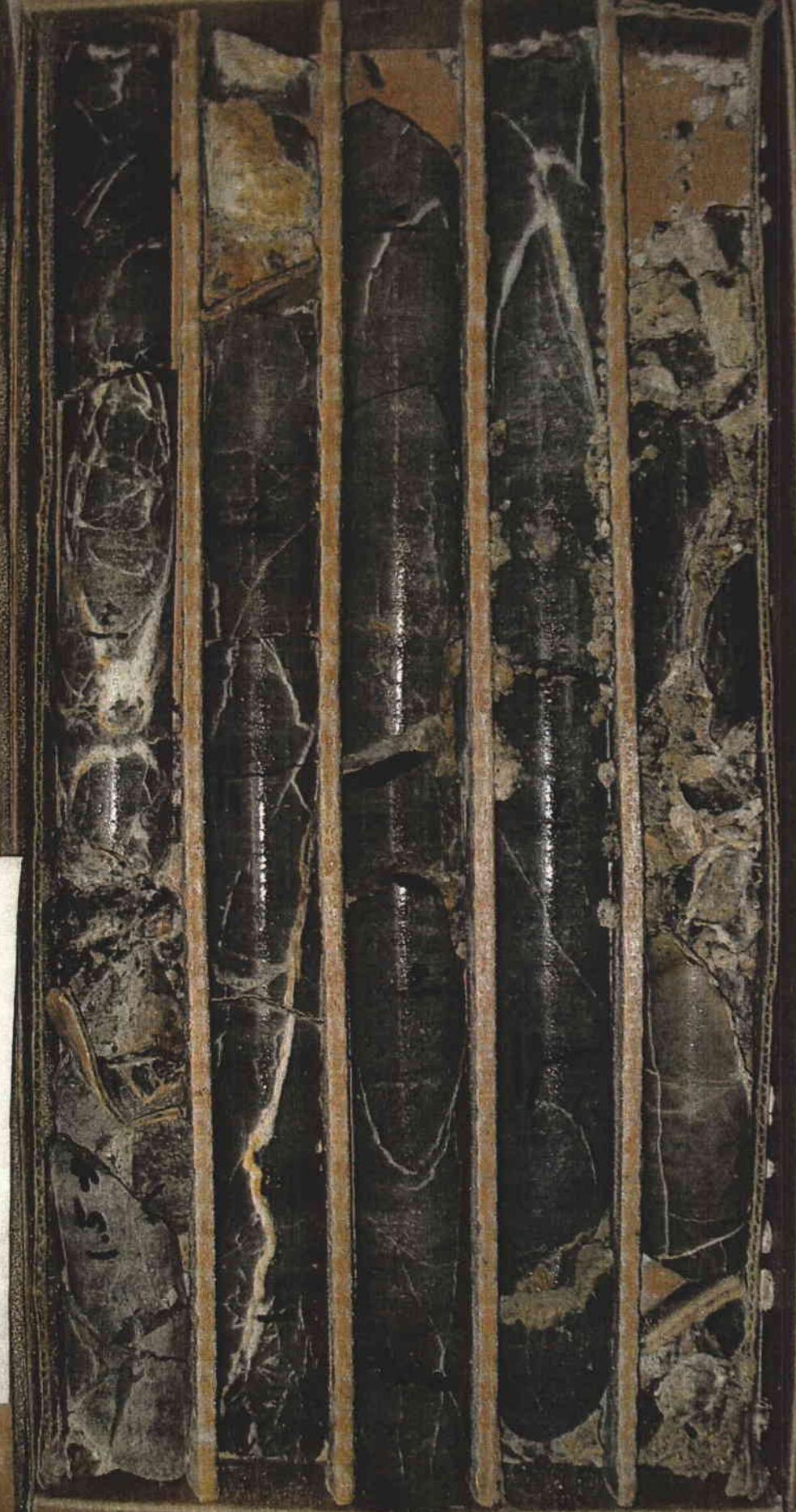
HOLCIM  
Poverty Point  
DH - 06 - 03

**YEAR**

**10**

**TO**

HOLCIM - Poverty Point  
Drill Hole - D6 - D4  
Box 1 0 - 11.5



HOLCIM - Poverty Point  
Drill Hole - D6 - 04

Box 2 11.5-20.3



HOLCOM - Poverty Point  
Drill Hole - D6 - D4

Inv 2203 - 29.4



R

TOP

HOLCIM - Poverty Point  
Drill Hole - D6 - D4

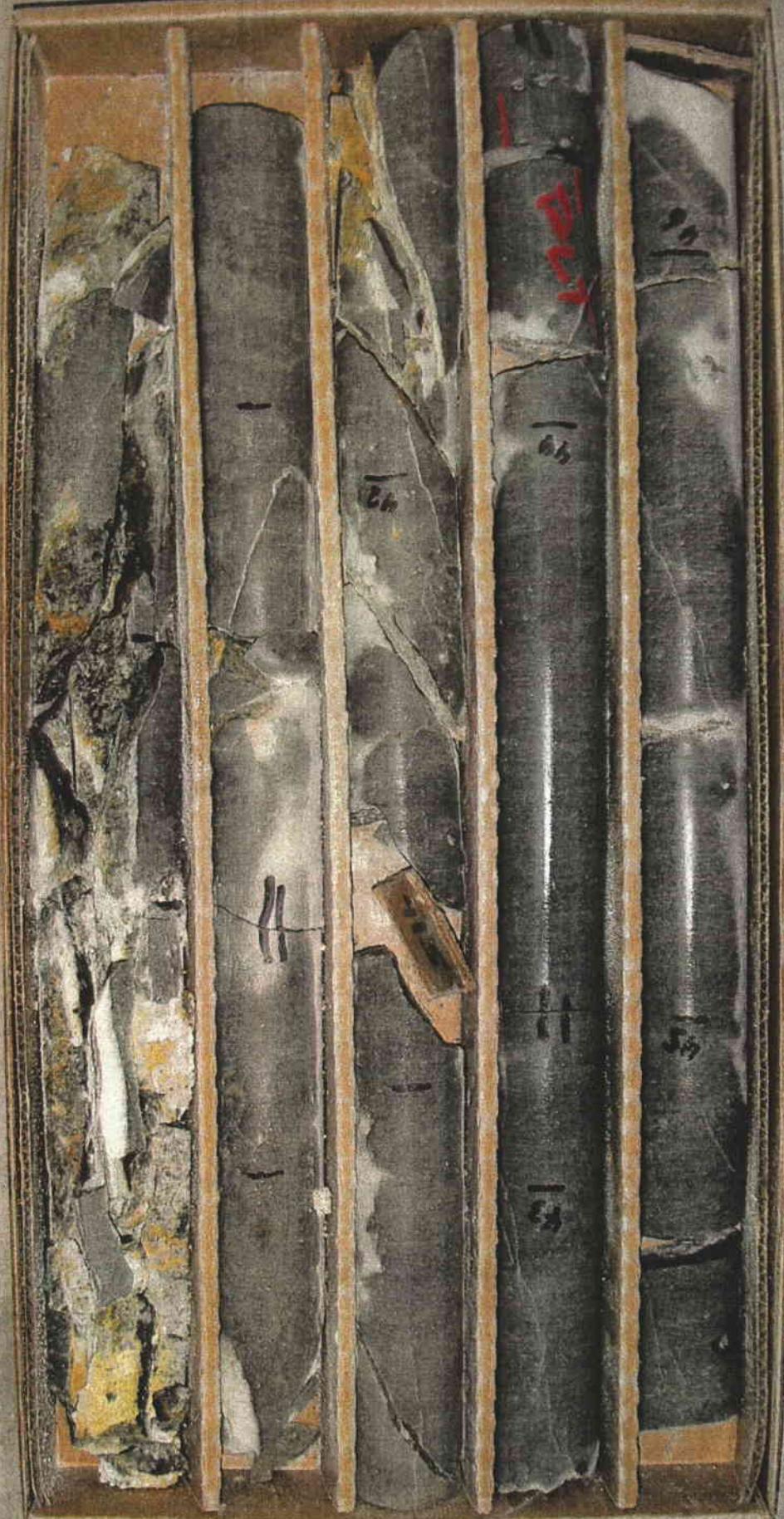
Box 4 29.4. 37.9



HOLCOM - Poverty Point  
Drill Hole - D6 - D4

Box 5 37.9 - 46.3

ON



HOLCIM - Poverty Point  
Drill Hole - D6 - D4

Box 6 46.3 - 55.4

HOLCIM - Poverty Point  
Drill Hole - D6 - 04

Box 7 55.4 - 64.5

h5

57

09

HOLCIM - Poverty Point  
Drill Hole - D6 - 04

Box 8 64.5 - 73.6

HOLCIM - Poverty Point  
Drill Hole - D6 - D4

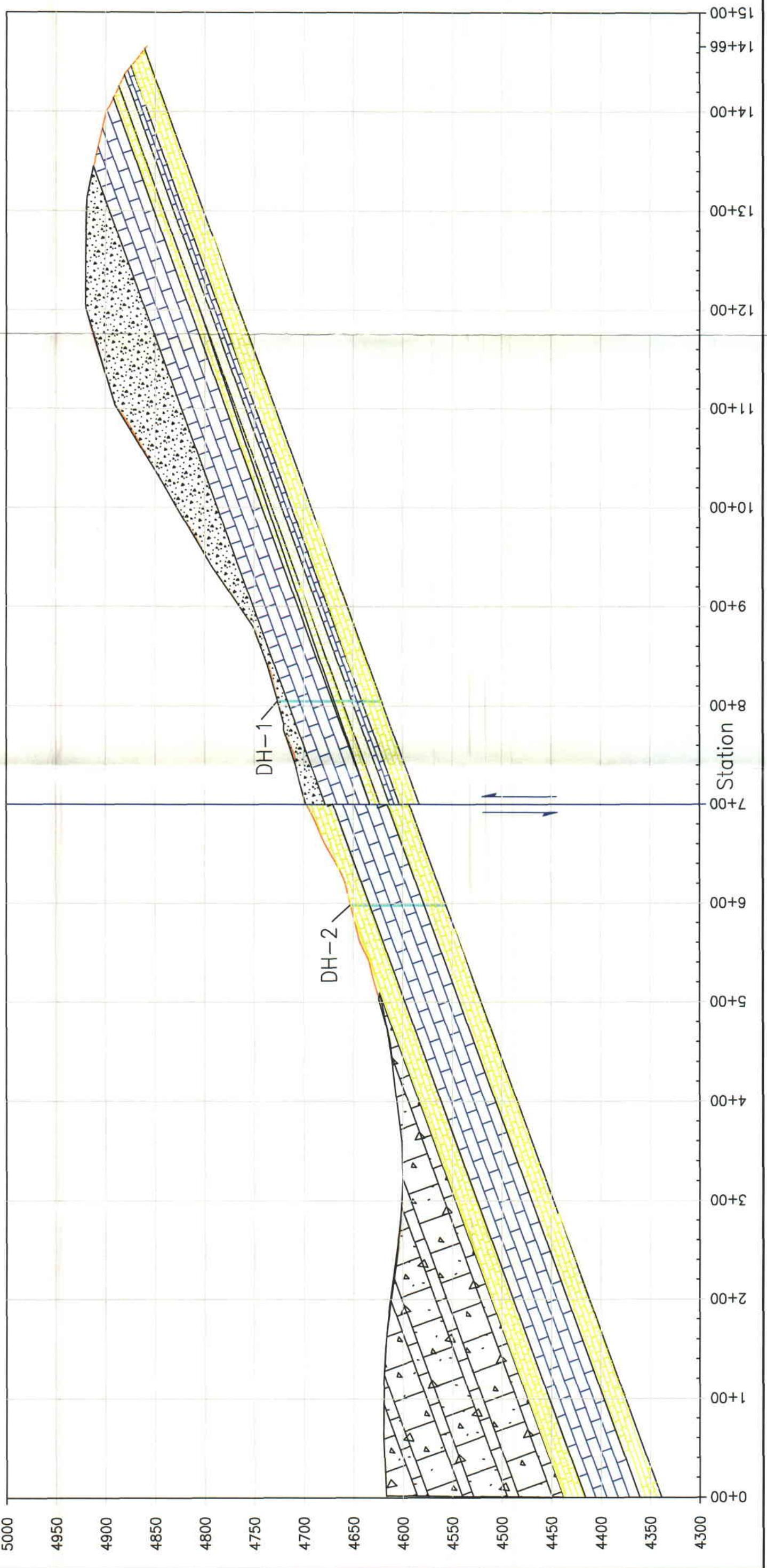
Box 9 736.82.4

HOLCIM - Poverty Point  
Drill Hole - D6 - D4

Box 10 82.4-90.5

**APPENDIX C**

**Cross-Sections**

**A'****A**

LEGEND					
				ALUVIUM	
				TALUS (CONGLOMERATE)	
				HIGH PURITY Ls	
				SILICEOUS Ls	
				SANDSTONE	

FILE: H:\HOLCIM\Drafting\FigCs\Cross-Sections.dwg

REV	DATE	DESCRIPTION	BY	CHKD APRV'D

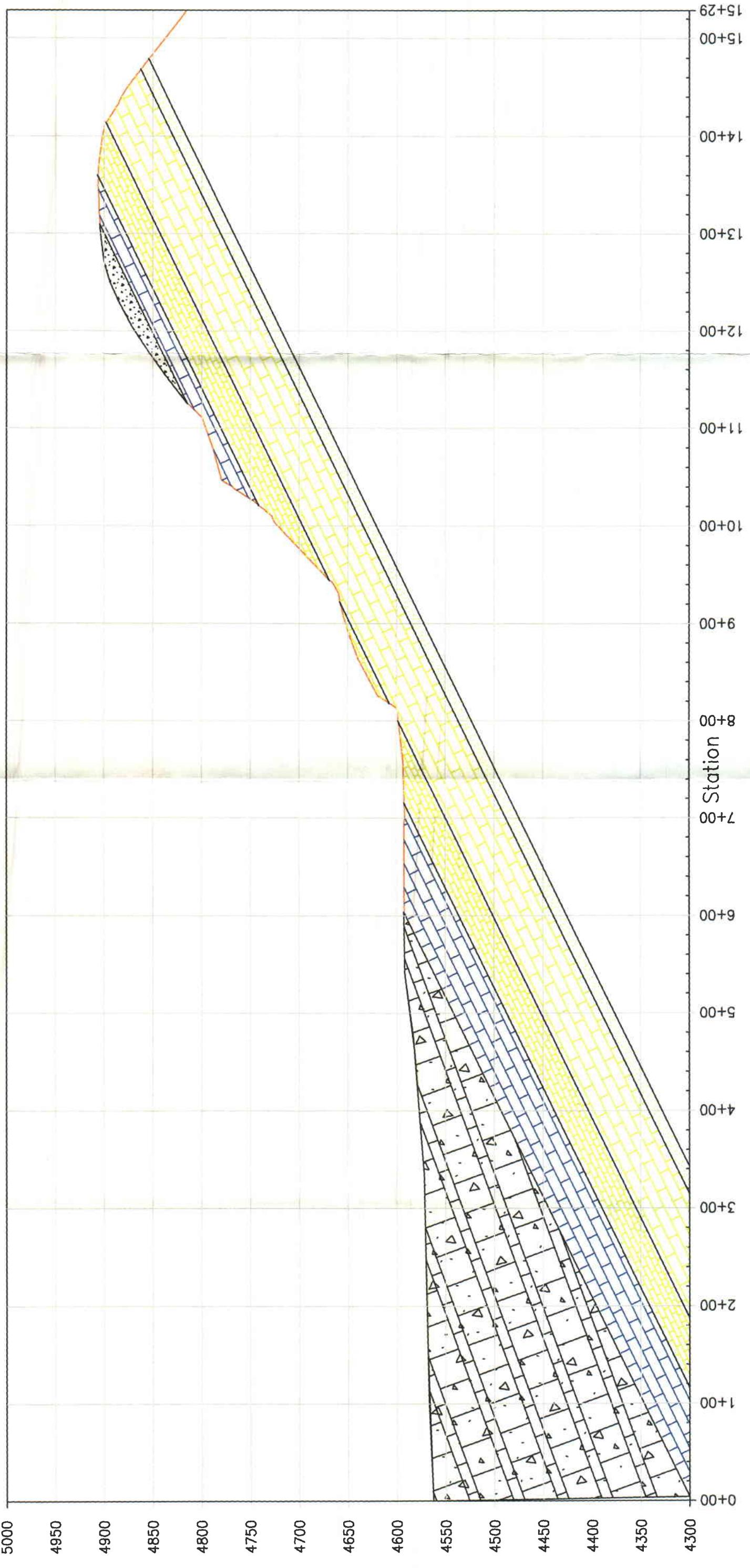
DRAWN BY: HAS DATE: 01-May-06 REV: 00

NORWEST FigCs Cross-Sections.dwg

Fig. C-1

FIGURE C-1  
CROSS-SECTION A-A'SCALE: 1" = 100'  
0 50 100 200 300

二



LEGEND

- ALUVIUM
- TALUS (CONGLOMERATE)
- HIGH PURITY ls
- SILICEOUS ls
- SANDSTONE

H:\HOLCIM\Drafting\FIGCs Cross-Sections.dwg

REV	DATE	DESCRIPTION	BY	CHKD	APRV'D
-	-	-	-	-	-

**HOLCIM**

POVERTY POINT

**FIGURE C-2**

**CROSS-SECTION B-B'**

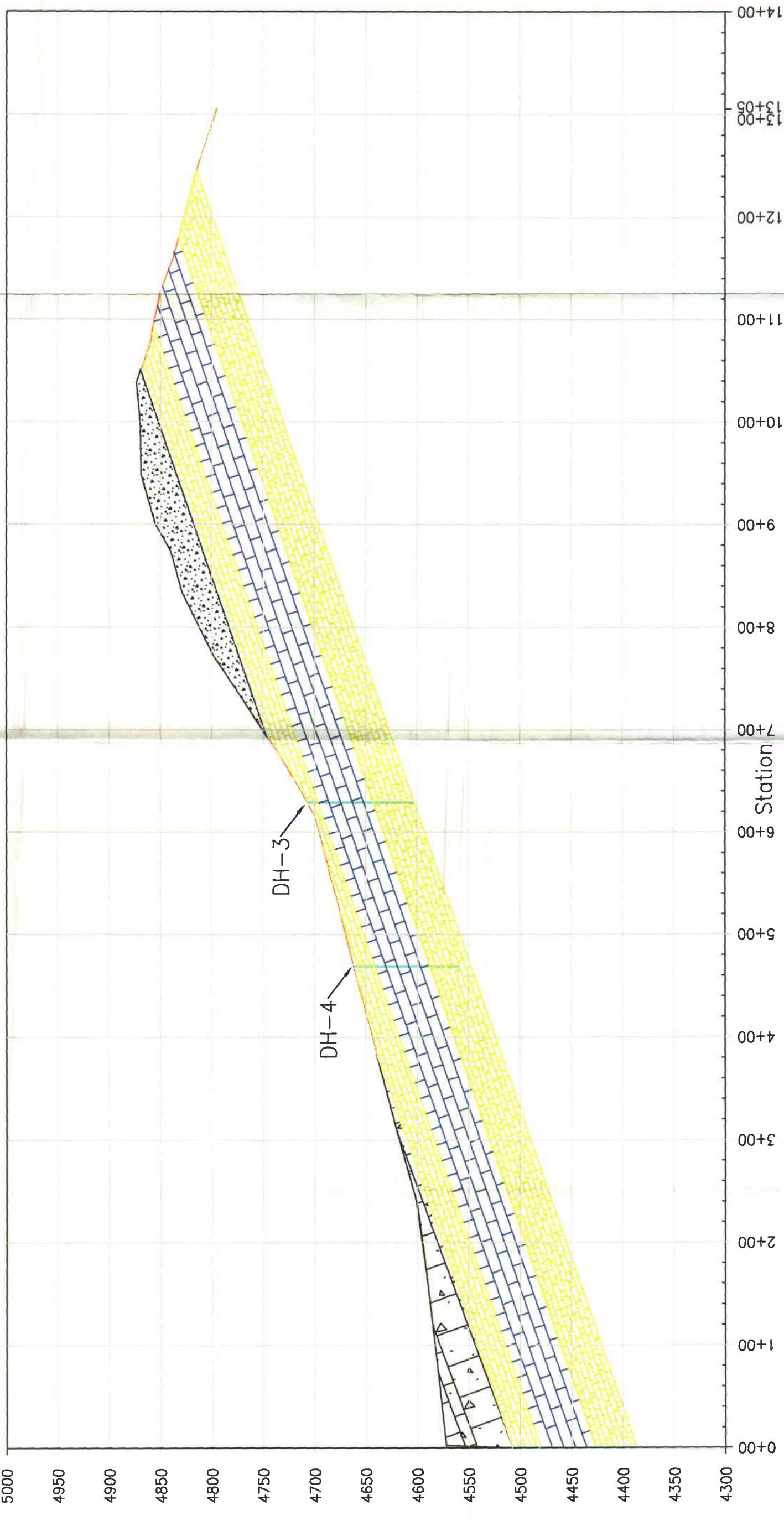
**FIGURE C-2**  
**CROSS-SECTION B-B'**

**FIGURE C-2**

FIG

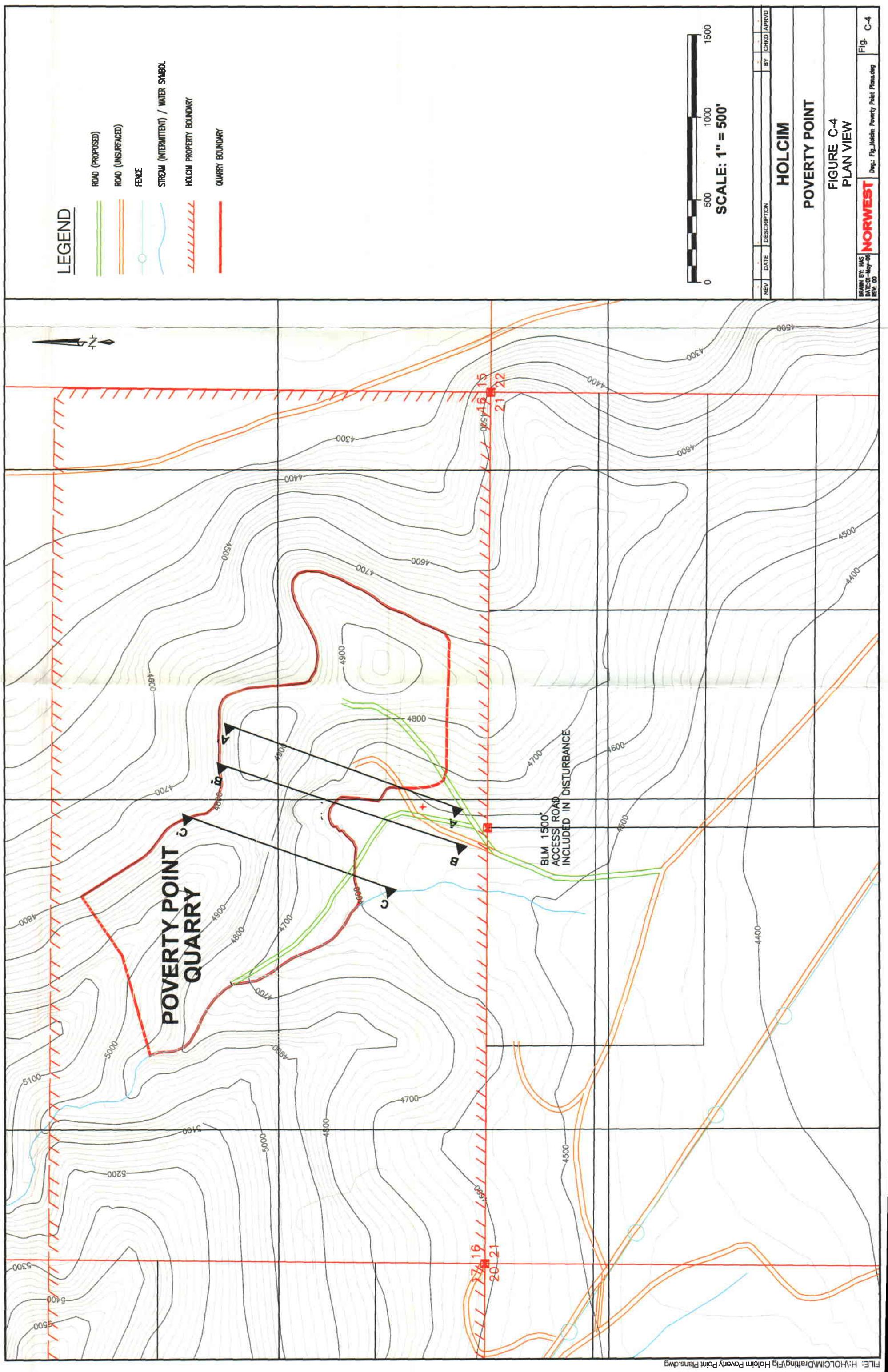
300

SCALE: 1" = 100'

**C'****C**

REV	DATE	DESCRIPTION	BY	CHKD	APRVD

**HOLCIM****POVERTY POINT****CROSS-SECTION C-C'****NORWEST****FIGURE C-3****Dwg.: FigCs Cross-Sections.dwg****Fig. C-3**



**APPENDIX D**

**Site Inspection Memo**

---

# NORWEST

C O R P O R A T I O N

## MEMORANDUM

---

**TO:** MIKE TOELLE  
**FROM:** GARY M. STUBBLEFIELD  
**SUBJECT:** POVERTY POINT SITE INSPECTION  
**DATE:** JUNE 9, 2005  
**JOB #:** 05-3213  
**CC:** KIRK WEBER, KEN GEORGE, LANCE STEPHENS

---

Dear Mike,

I met Paul Johnson and Brian Ward at Rowley junction west of Salt Lake City at 9:30am, and followed them to the Poverty Point Quarry, arriving approximately 10:00am. There was no mining activity, nor equipment at the Quarry. Brian provided some maps of the quarry and we briefly discussed the activities I would be conducting on this inspection. Paul and Brian then departed to return to the Devil's Slide Quarry.

Over the next two hours I walked throughout the quarry taking photos, measuring bedding plane dips, looking for joint patterns, faults and weak zones. My observations are as follows:

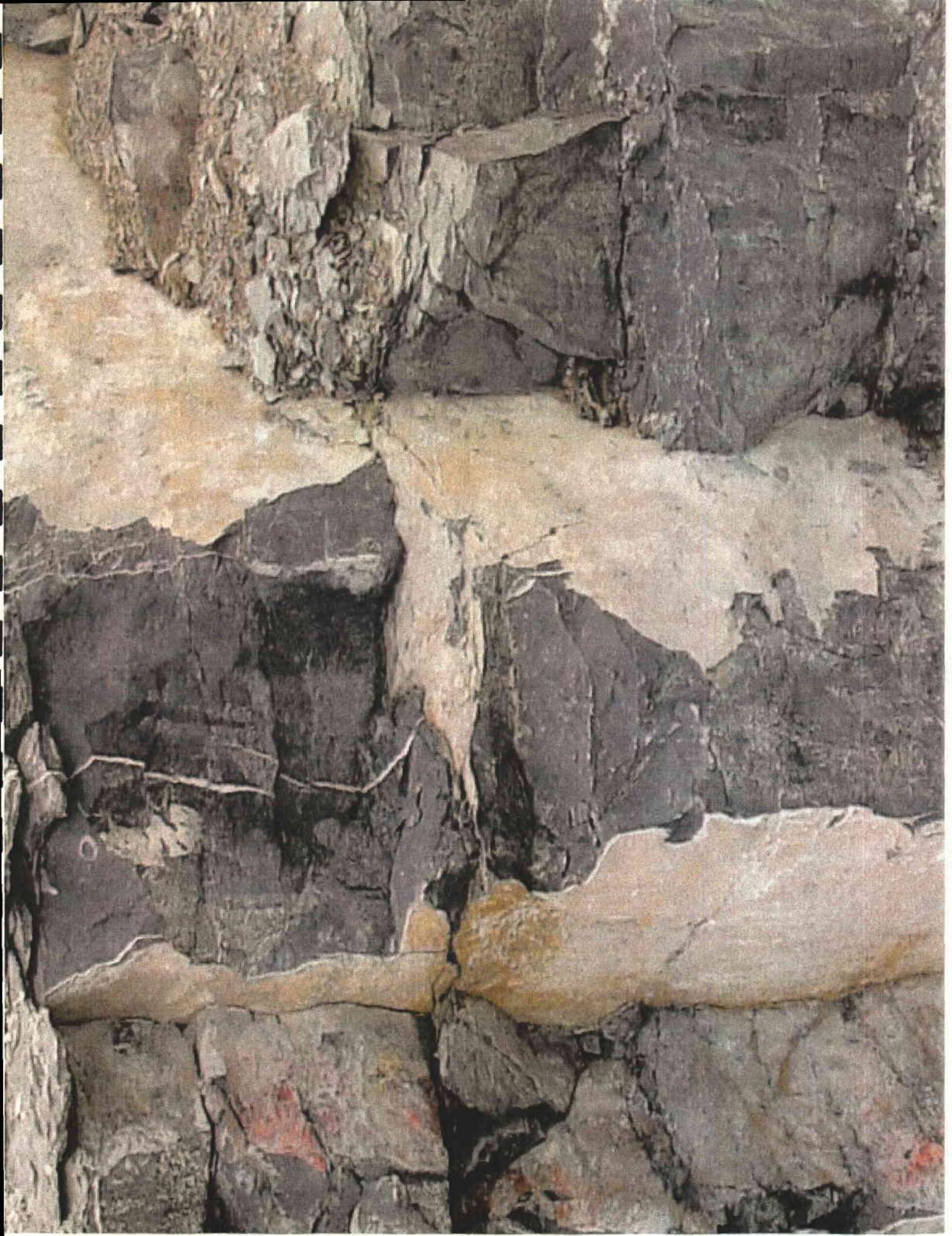
- The quarry has had considerable previous mining activity, evidenced by the existence of several (5 to 6) benches from the pit floor at the 4550 ft. elevation, to a saddle near the top of the mountain at the 4850 ft. elevation. It appears the pit was developed from the bottom up, leaving a nearly 300-foot highwall on the north end. The rock appeared to be competent, but exhibited jointing patterns (see photo 36). These joints had thin calcite deposits and were oriented N22°E in one location, and N33°E and N40°W in another location (see photo 43)
- Beds dip to the south at 25 to 35 degrees into the quarry. It appears that the quarry was opened on the south and developed to the north. This left the south end daylighted to the natural terrain, while the north pit wall rises about 300 hundred feet to the upper reaches of the mountain. I observed a couple of vertical joints that had been in-filled with calcite deposits and a thin (1 inch) layer of sandy clay beneath a thick limestone layer (photo 42). I observed only one small area of possible faulting (photo 37).

- Near the upper reaches of the quarry, I observed cobblestone, rounded by wave action along the ancient Lake Bonneville shoreline. There was one wet area on the highwall associated with alluvial material overlying the limestone (photo 44). This could potentially weaken the immediate area, but did not seem to impact the broader highwall.
- The north pit wall appeared ragged, i.e., a few overhanging rocks and a jagged face (Photos 33 and 41). I did not observe any actual slope failures, but the joint pattern coupled with the south dipping beds suggest that slope failures could occur if the quarry wall is advanced further northward without un-weighting from the top.
- There was evidence of blasting at the quarry (nonel blasting cord and spent surface delays), which is to be expected. Blasting practices can contribute to slope instability and this should be investigated to determine if improvements could be made.

I will send the photos along with this inspection report to our geotechnical group in Vancouver for their review. From this initial inspection and observations, Norwest will prepare our proposal to conduct a proper geotechnical assessment of the Poverty Point Quarry, including recommendations for improving pit wall stability. We should have this proposal completed by mid-next week.

Sincerely,

Gary Stubblefield, P.E.  
Vice President



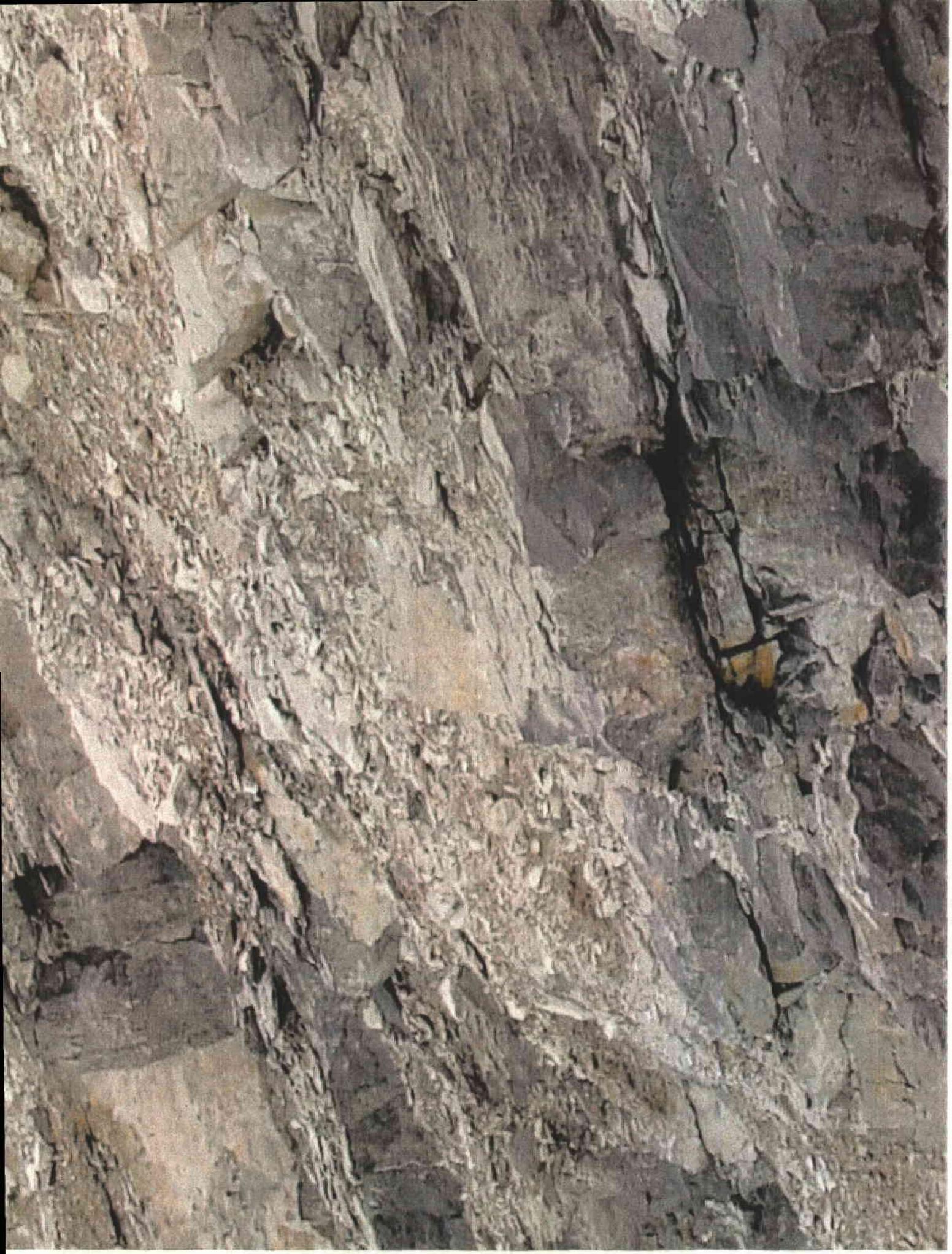














## **APPENDIX E**

### **Load Tests**

DESIGN BY

DATE 2.16.06 CHECKED BY \_\_\_\_\_ SHEET NO. 1

PROJECT HOLCIMJOB NO. 3215SUBJECT POINT LOAD TESTING

HOLE 06-003

DEPTH psf

15 1165

25 1270

36 960

52 960

55' 1010

64<sup>15</sup>' 1210

X 70 1395

6 1500 psf

88 850

DESIGN BY \_\_\_\_\_ DATE 2/16/06 CHECKED BY \_\_\_\_\_ SHEET NO. 21PROJECT HOLCOM JOB NO. 3213SUBJECT POINT LOAD TESTING

HOLE  
06-04

<u>DEPTH</u>	<u>PSI</u>
16 <sup>5</sup>	1470
23 <sup>5</sup>	900
35	615
44 <sup>5</sup>	<u>1280</u>
58 <sup>5</sup>	1130
66	960
74 <sup>5</sup>	665
82 <sup>5</sup>	1175



FAILURE  
BREAK      SOME CALCITE  
VEINING - DONT  
ON IT

DESIGN BY \_\_\_\_\_ DATE 2/16/06 CHECKED BY \_\_\_\_\_ SHEET NO. 31PROJECT HOLCOM JOB NO. 3213SUBJECT POINT LOAD TESTINGHOLE ID06 - 02

<u>DEPTH</u>	<u>PSI</u>
16'	1440 DID NOT FAIL SCALES
18'	1390
✓ 26 <sup>5</sup>	1350
35 <sup>5</sup>	1150
46	1395
57	950
68	1250
X 74 <sup>25</sup>	1105
80	1210
92	1340
<del>105<sup>5</sup></del>	
X 111	1270
119 <sup>5</sup>	105 - BROKE FEQ ON BREAK
<del>125<sup>5</sup></del>	EST 1650
X 137	1500



PROJECT: Norwest Corporation  
LOCATION:  
SAMPLED BY: AS RECEIVED  
AUTHORIZED BY:  
NOM. MAX SIZE AGGREGATE:  
CONTRACTOR:  
SUPPLIER:  
MATERIAL:

JOB NO: 6-819-002650  
WORK ORDER NO: T5585  
DATE PLACED:  
DATE TESTED:  
DATE REC'D:  
PSI @ 28 DAYS:  
PRODUCT CODE:

UNCONFINED COMPRESSION - ROCK CORES  
ASTM D2938

LAB NO.	1	2	3	4	5
DATE TESTED	3/1/06	3/1/06	3/1/06	3/1/06	03/010/6
AGE (DAYS)					
LENGTH RECEIVED (IN)					
LENGTH BEFORE CAP (IN)					
LENGTH AFTER CAP (IN)	3.6	3.6	3.6	3.6	3.6
DIAMETER (IN)	1.80	1.80	1.80	1.80	1.80
AREA (IN^2)	2.54	2.54	2.54	2.54	2.54
LOAD (LBS.)	19,570	29,480	46,450	29,780	33,570
STRESS (PSI)	7,691	11,585	18,254	11,703	13,192
L/D	2.00	2.00	2.00	2.00	2.00
CORRECTED STRESS (PSI)	7,690	11,580	18,250	11,700	13,190

NOTE:

LOCATION: 5585-1, 3213 Hole 06.03, 25' - 26'  
5585-2, 3213 Hole 06.02, 137.5'  
5585-3, 3213 Hole 06.02, 26.75'  
5585-4, 3213 Hole 06.04, 35.2'  
5585-5, 3213 Hole 06.03, 70.3'

Reviewed By:

Derek S. Anderson



PROJECT: Norwest Corporation  
LOCATION:  
SAMPLED BY: AS RECEIVED  
AUTHORIZED BY:  
NOM. MAX SIZE AGGREG:  
CONTRACTOR:  
SUPPLIER:  
MATERIAL:

JOB NO: 6-819-002650  
WORK ORDER NO: T5585  
DATE PLACED:  
DATE TESTED:  
DATE REC'D:  
PSI @ 28 DAYS:  
PRODUCT CODE:

**UNCONFINED COMPRESSION - ROCK CORES**  
**ASTM D2938**

LAB NO.	6	7	8	9	10
DATE TESTED	3/1/06	3/1/06	3/1/06	3/1/06	03/010/6
AGE (DAYS)					
LENGTH RECEIVED (IN)					
LENGTH BEFORE CAP (IN)					
LENGTH AFTER CAP (IN)	3.6	3.6	3.6	3.6	3.6
DIAMETER (IN)	1.80	1.80	1.80	1.80	1.80
AREA (IN^2)	2.54	2.54	2.54	2.54	2.54
LOAD (LBS.)	31,220	21,780	26,990	41,130	25,290
STRESS (PSI)	12,269	8,559	10,606	16,163	9,938
L/D	2.00	2.00	2.00	2.00	2.00
CORRECTED STRESS (PSI)	12,270	8,560	10,610	16,160	9,940

NOTE:

LOCATION: 5585-6, 3213 Hole 06.02, 121.0'  
5585-7, 3213 Hole 06.03, 48.5'  
5585-8, 3213 Hole 06.04, 167.0'  
5585-9, 3213 Hole 06.04, 183'  
5585-10, 3213 Hole 06.02, 74'

Reviewed By:

DeeDee Jackson



PROJECT: Norwest Corporation  
LOCATION:  
SAMPLED BY: AS RECEIVED  
AUTHORIZED BY:  
NOM. MAX SIZE AGGREG:  
CONTRACTOR:  
SUPPLIER:  
MATERIAL:

JOB NO: 6-819-002650  
WORK ORDER NO: T5585  
DATE PLACED:  
DATE TESTED:  
DATE REC'D:  
PSI @ 28 DAYS:  
PRODUCT CODE:

---

**UNCONFINED COMPRESSION - ROCK CORES**  
**ASTM D2938**

---

LAB NO. 11

DATE TESTED 3/1/06

AGE (DAYS)

LENGTH RECEIVED (IN)

LENGTH BEFORE CAP (IN)

LENGTH AFTER CAP (IN) 3.6

DIAMETER (IN) 1.80

AREA (IN<sup>2</sup>) 2.54

LOAD (LBS.) 35,820

STRESS (PSI) 14,076

L/D 2.00

CORRECTED STRESS (PSI) 14,080

NOTE:

LOCATION: 5585-11, 3213 Hole 06.04, 59.2'

Reviewed By:

Derek S. Anderson

**APPENDIX F**

**Rock Strength Data**

Classification Parameters										Ratings									
Depth	From (ft)	To (ft)	Interval	Rock Strength Load(MPa)	RQD%	Joint Count	Joint Spacing(ft)	Joint Condition	Water Condit	Rock Strength	Joint Spacing(m)	Joint Condition	Water Condit	Rock Strength	Joint Spacing(m)	Joint Condition	Water Condit	Rock Strength	
0	0	3	3	1	2	32	NA	#VALUE!	#VALUE!	damp	4	NA	NA	NA	10	#VALUE!	LS	NA	
3	3	4	1	2	0	2	18	NA	#VALUE!	damp	4	NA	NA	NA	10	#VALUE!	LS	NA	
6	4	6	2	2	2	2	52	NA	#VALUE!	damp	4	4	3	#VALUE!	0	0	10	#VALUE!	
8	6	8	2	2	2	2	0	NA	#VALUE!	damp	4	4	13	#VALUE!	0	0	10	#VALUE!	
11	8	11	3	2	2	2	22	NA	#VALUE!	damp	4	4	3	#VALUE!	0	0	10	#VALUE!	
15	11	15	2.5	3	9.9	32	NA	#VALUE!	#VALUE!	damp	4	4	3	#VALUE!	0	0	10	#VALUE!	
16.5	13.5	16.5	3	4.5	9.6	76	11	0.41	0.12	damp	12	12	8	#VALUE!	0	0	10	47 LS	
21	21	26	5	10	79	10	10	0.50	0.15	damp	12	12	8	#VALUE!	0	0	10	47 LS	
28	28	31	5	9.3	98	1	5.00	1.52	damp	12	20	15	NA	57 LS	0	0	10	42 LS	
31	31	36	5	7.9	88	6	0.83	0.25	damp	12	17	10	0	49 LS	0	0	10	50 LS	
36	36	39	3	8	0	20	0.15	0.05	damp	12	3	5	0	30 LS	0	0	10	46.5 LS	
39	39	43	4	9	35	18	0.20	0.06	damp	12	8	8	0	38 LS	0	0	10	39.5 LS	
43	43	47	4	4.5	9.6	38	18	0.22	0.07	damp	12	6	8	0	10	0	0	10	36 LS
47	47	52	5	7	0	20	0.25	0.08	damp	12	3	8	0	33 LS	0	0	10	38.4 LS	
52	52	57	5	6.6	8	8	0.63	0.19	damp	12	3	8	0	10	0	0	10	35 LS	
57	57	62	5	6.6	11	25	0.20	0.06	damp	12	3	8	0	10	0	0	10	33 LS	
62	62	67	5	7	95	15	0.33	0.10	damp	12	20	8	0	10	0	0	10	50 LS	
67	67	72	5	8.6	63	1	5.00	1.52	damp	12	13	15	0	10	0	0	10	45 LS	
72	72	77	5	7.6	70	5	1.00	0.30	damp	12	13	10	0	10	0	0	10	43 LS	
77	77	81.5	4.5	8.3	49	10	0.45	0.14	damp	12	8	8	0	10	0	0	10	38 LS	
81.5	81.5	86	4.5	5.7	9	0.50	0.15	damp	12	13	8	0	11	44 LS	0	0	10	38 LS	
86	86	87	1	8	28	8	0.13	0.04	damp	12	8	5	0	12	37 LS	0	0	10	36 LS
87	87	92	5	9.2	15	22	0.23	0.07	damp	12	3	8	0	13	36 LS	0	0	10	36 LS
92	92	97	5	9.2	19	25	0.20	0.06	damp	12	3	8	0	14	37 LS	0	0	10	46 LS
97	97	100	3	8	0	15	0.20	0.06	damp	12	3	8	0	15	38 LS	0	0	10	46 LS
100	100	103	3	6	0	10	0.30	0.09	damp	12	3	8	0	16	39 LS	0	0	10	46 LS
103	103	106	5	8	18	20	0.25	0.08	damp	12	3	8	0	17	40 LS	0	0	10	46 LS
106	106	113	5	8.8	38	20	0.25	0.08	damp	12	8	8	0	18	46 LS	0	0	10	40 LS
113	113	118	5	6	26	25	0.20	0.06	damp	12	8	8	0	19	47 LS	0	0	10	44 LS
118	118	121	3	2.8	23	20	0.15	0.05	damp	7	3	5	0	20	35 LS	0	0	10	30 LS
121	121	125.5	4.5	11.4	33	20	0.23	0.07	damp	15	8	8	0	21	52 LS	0	0	10	8.4 LS
125.5	125.5	130.5	5	11.4	18	15	0.33	0.10	damp	15	3	8	0	22	48 LS	0	0	10	51.3 LS
130.5	130.5	135	4.5	10	21	20	0.23	0.07	damp	12	3	8	0	23	46 LS	0	0	10	57 LS
135	135	138	3	10.3	18	20	0.15	0.05	damp	15	3	5	0	24	47 LS	0	0	10	45 LS

Load	8	Load	4-10
Avg MI	9		
Avg UCS	80	Range UCS	53-123
Avg RMR	42		



Hole No. DH 06-04

Depth From (ft)	To (ft)	Classification Parameters						Ratings						
		Interval	Rock Strength (Point Load) [MPa]	RQD%	Joint Count	Joint Spacing (ft)	Joint Spacing (m)	Joint Condition	Rock Strength	RQD	Joint Spacing	Joint Condition	Rock Point Load	Weight RMR
0	1.5	1.5	2	0	18	0.08	0.03	weathered	damp	3	5	20	10	NA
1.5	5	3.5	6	60	7	0.50	0.15	weathered	damp	12	13	8	20	10
5	10	5	6	57	12	0.42	0.13	slight rough	damp	12	13	8	25	10
10	11.5	4	22	5	0.30	0.09	slight rough	damp	7	3	8	25	10	
11.5	16.5	5	10	60	14	0.36	0.09	smooth	damp	15	13	8	10	10
16.5	21.5	5	8	51	18	0.28	0.08	smooth	damp	12	13	8	10	10
21.5	26.5	5	6	35	13	0.38	0.12	slight rough	damp	12	8	8	25	10
26.5	31.5	5	6	83	8	0.63	0.19	slight rough	damp	12	17	8	25	10
31.5	36.5	5	4	76	10	0.50	0.15	slight rough	damp	12	17	8	25	10
36.5	41.5	5	7	39	10	0.50	0.15	slight rough	damp	12	8	8	25	10
41.5	46.5	5	7	79	7	0.71	0.22	slight rough	damp	12	17	10	25	10
46.5	51.5	5	6	64	10	0.50	0.15	smooth	damp	12	13	8	10	10
51.5	56.5	5	5	41	10	0.50	0.15	smooth	damp	12	8	8	10	10
56.5	61.5	5	8	54	14	0.36	0.11	smooth	damp	12	13	8	10	10
61.5	66.5	5	7	51	11	0.45	0.14	slight rough	damp	12	13	8	25	10
66.5	71	4.5	6	78	11	0.41	0.12	smooth	damp	12	17	8	10	10
71	76	5	5	98	5	1.00	0.30	slight rough	damp	12	20	10	25	10
76	81	5	6	81	8	0.63	0.19	smooth	damp	12	17	8	10	10
81	85	4	8	95	8	0.50	0.15	smooth	damp	12	20	8	10	10
85	89	4	6	69	17	0.24	0.07	smooth	damp	12	13	8	10	10
89	95	1.5	6	0	1	1.50	0.46	smooth	damp	12	3	10	10	10

## Assumptions:

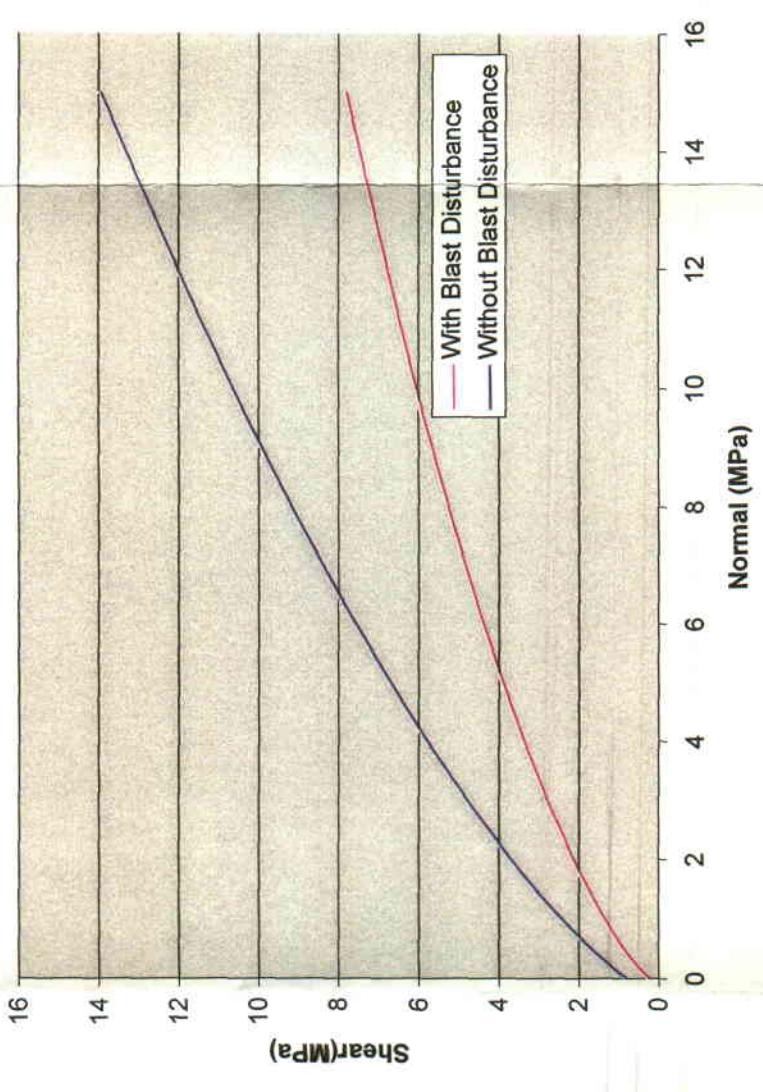
Where indicated in yellow, point load values were estimated based upon nearest depth test values, calculated RQD, rock strength (hammer blows), lithologic description, and/or joint condition. Groundwater conditions were estimated as damp - the influence of pore water pressures can be modeled within Slope-W Joint condition was estimated from geotechnical and geologic logs. All joints were planar or slightly curved, and rough or smooth. Calcite and trICl infilled most joints.

Avg Point Load	6	Range Point Load	4-10
Avg MI	9	Range UCS	53-123
Avg UCS	80		
Avg RMR	61		
Total RMR all holes	15962.5	Avg RMR (all holes)	55
Total Point Load (all holes)	2222.25	Avg POINT LOAD (all holes)	8
Interval (all holes)			290.5

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## **APPENDIX G**

### **Shear Strength Data**



0	286.8299
50	368.3667
100	443.3211
200	579.9582
400	819.9675
600	1032.442
800	1226.71
1000	1407.575
1500	1817.882
2000	2185.879
3000	2838.807
4000	3416.696
5000	3942.338
6000	4428.593
7000	4883.64
8000	5313.095
9000	5721.027
10000	6110.505
11000	6483.917
12000	6843.163
13000	7189.792
14000	7525.083
15000	7850.108

0	846.6859
50	948.9059
100	1047.187
200	1234.192
400	1579.699
600	1897.994
800	2196.459
1000	2479.482
1500	3136.403
2000	3739.969
3000	4837.848
4000	5833.696
5000	6756.418
6000	7622.8
7000	8443.734
8000	9226.819
9000	9977.635
10000	10700.44
11000	11398.58
12000	12074.75
13000	12731.16

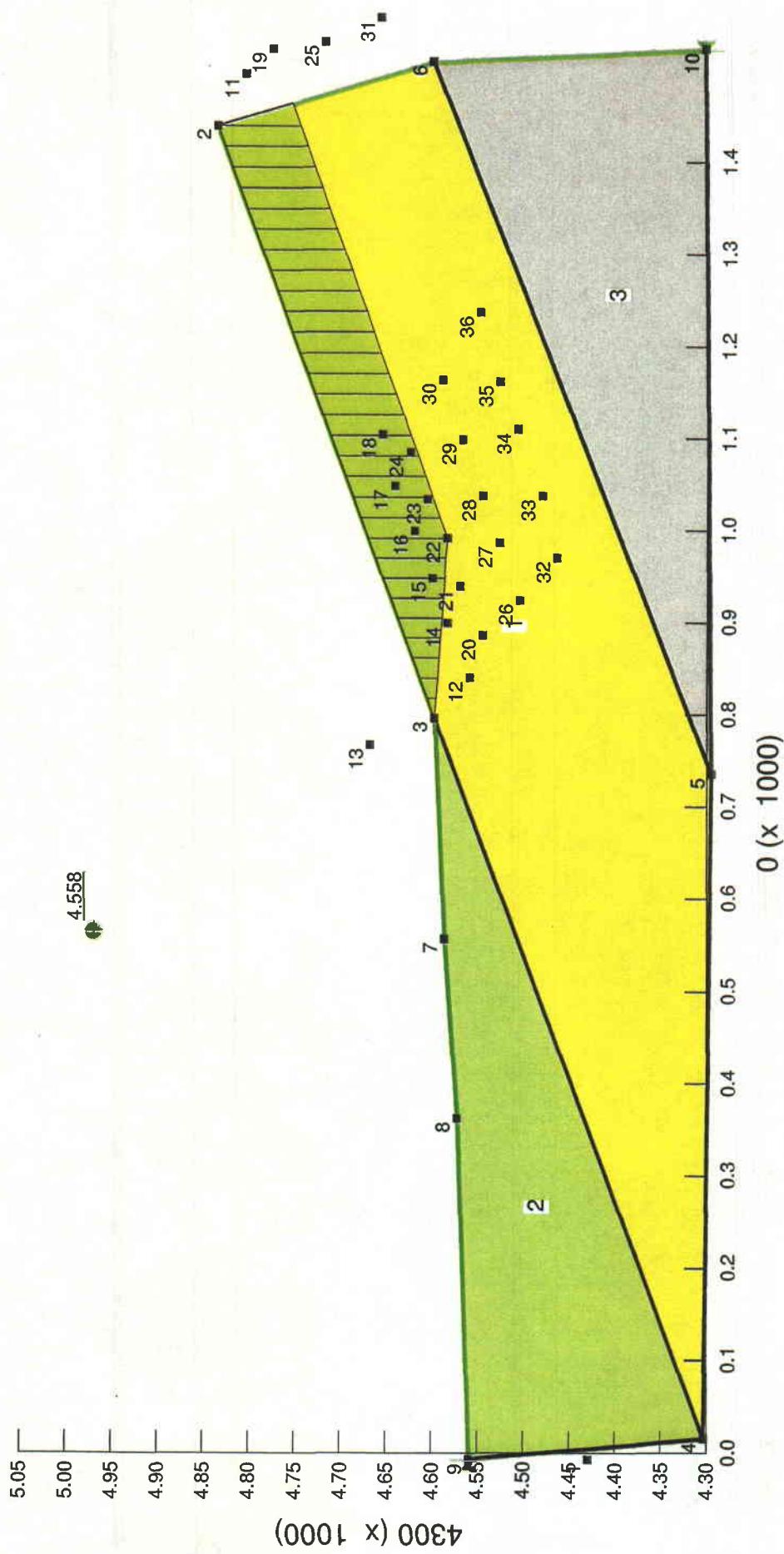
## **APPENDIX H**

### **Slope/W Analyses**

Name: 20 degrees.gsz

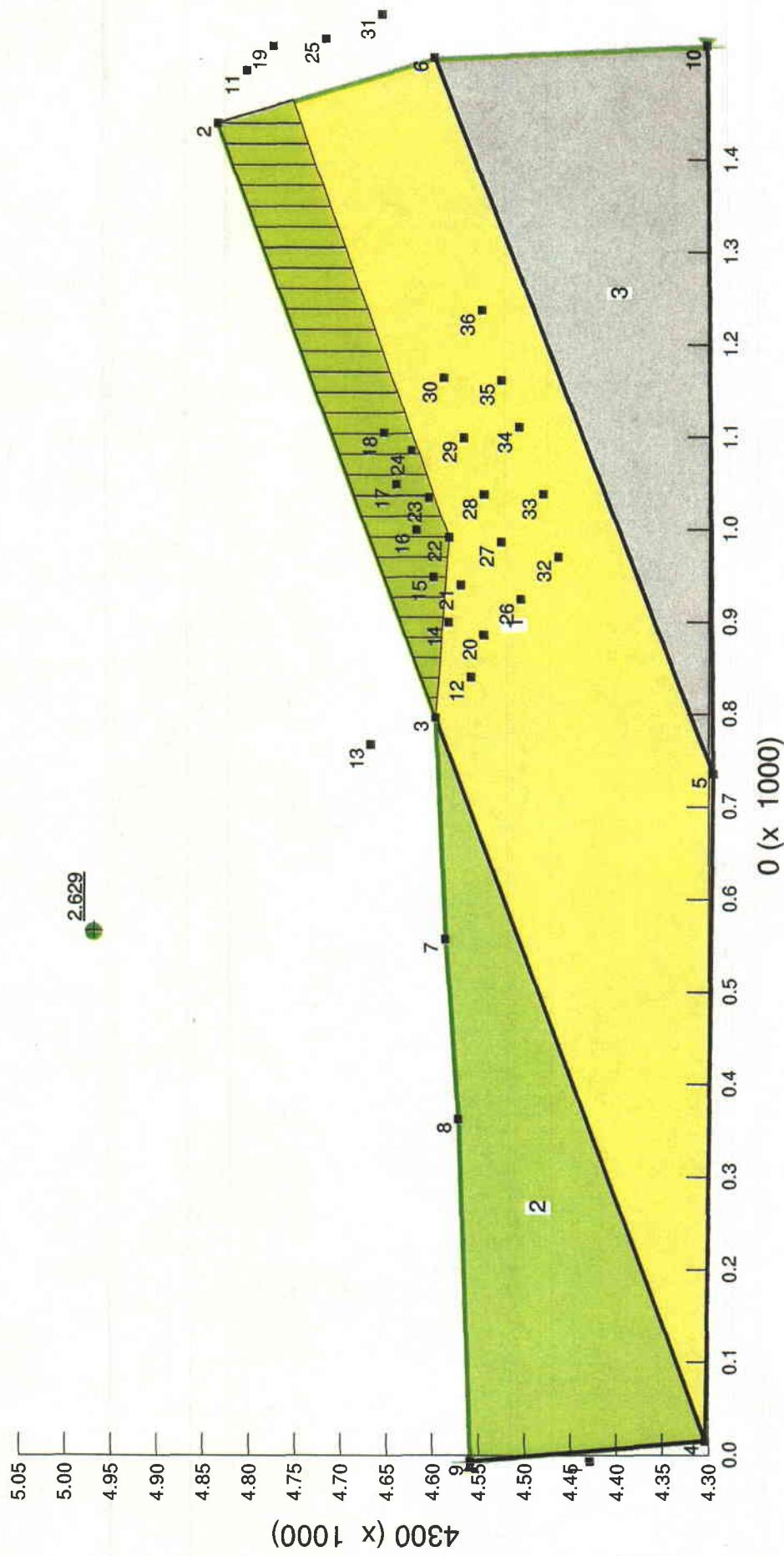
Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



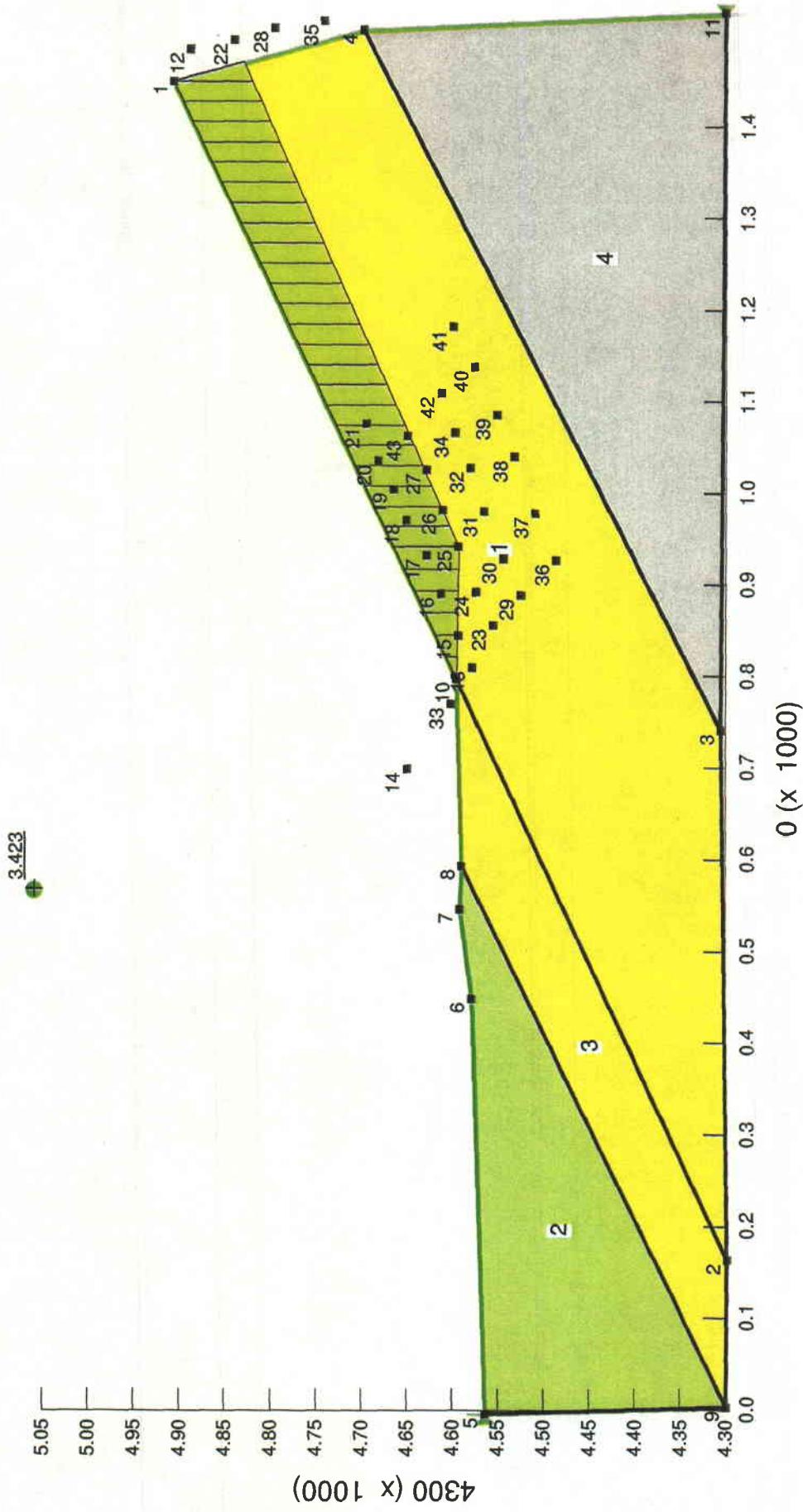
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 Material #: 1  
 Description: Blast Disturbed Limestone  
 Model: ShearNormalFn  
 Wt: 100  
 Strength Fn: 1  
 Material #: 2  
 Description: Undisturbed Limestone  
 Model: ShearNormalFn  
 Wt: 100  
 Strength Fn: 2

Material #: 3  
 Description: Alluvium  
 Model: MohrCoulomb  
 Wt: 100  
 Cohesion: 0  
 Phi: 45  
 Material #: 4  
 Description: Bedrock  
 Model: Bedrock



Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

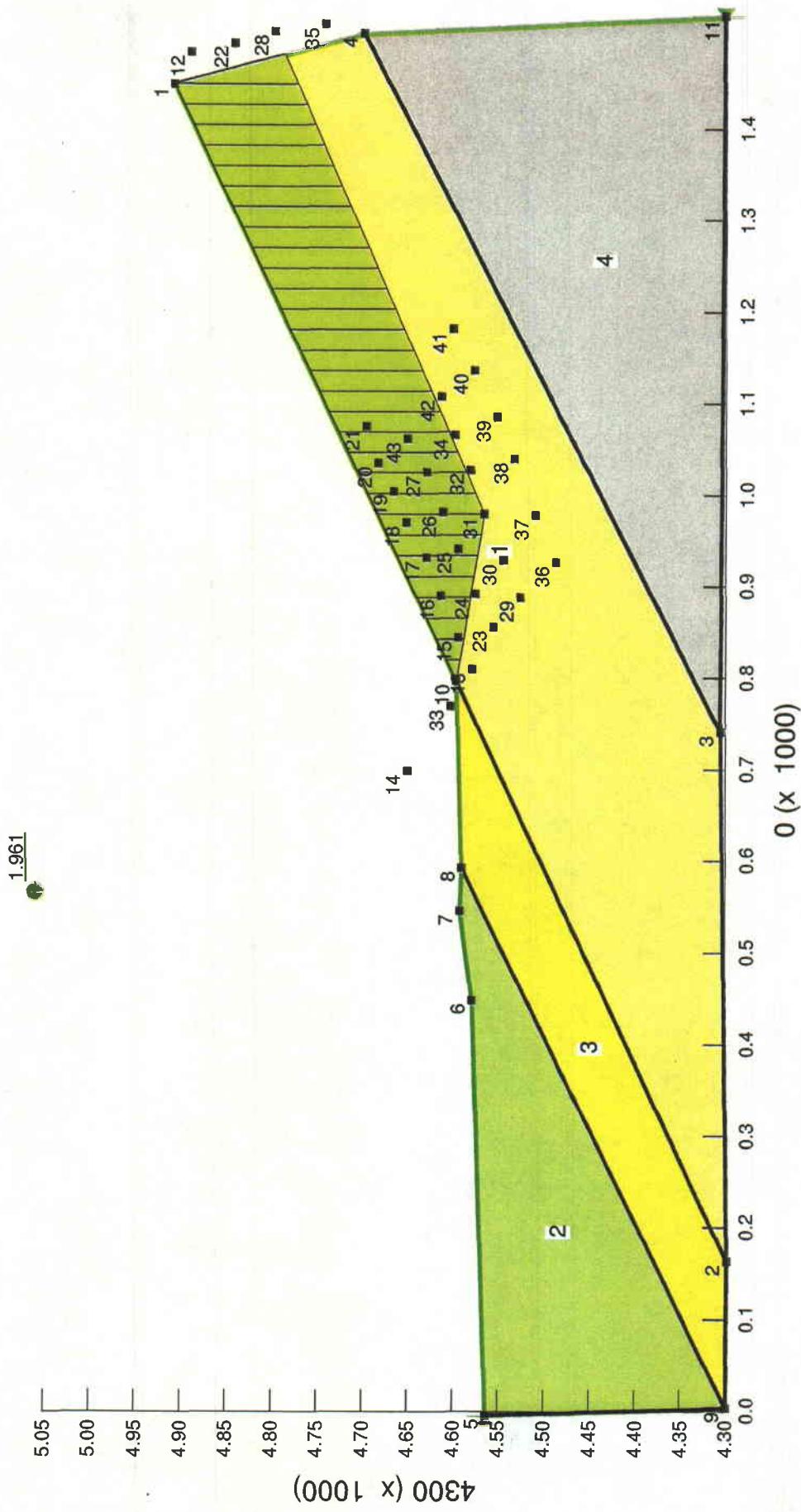
Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



Name: 25 degrees disturbed.gsz

Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

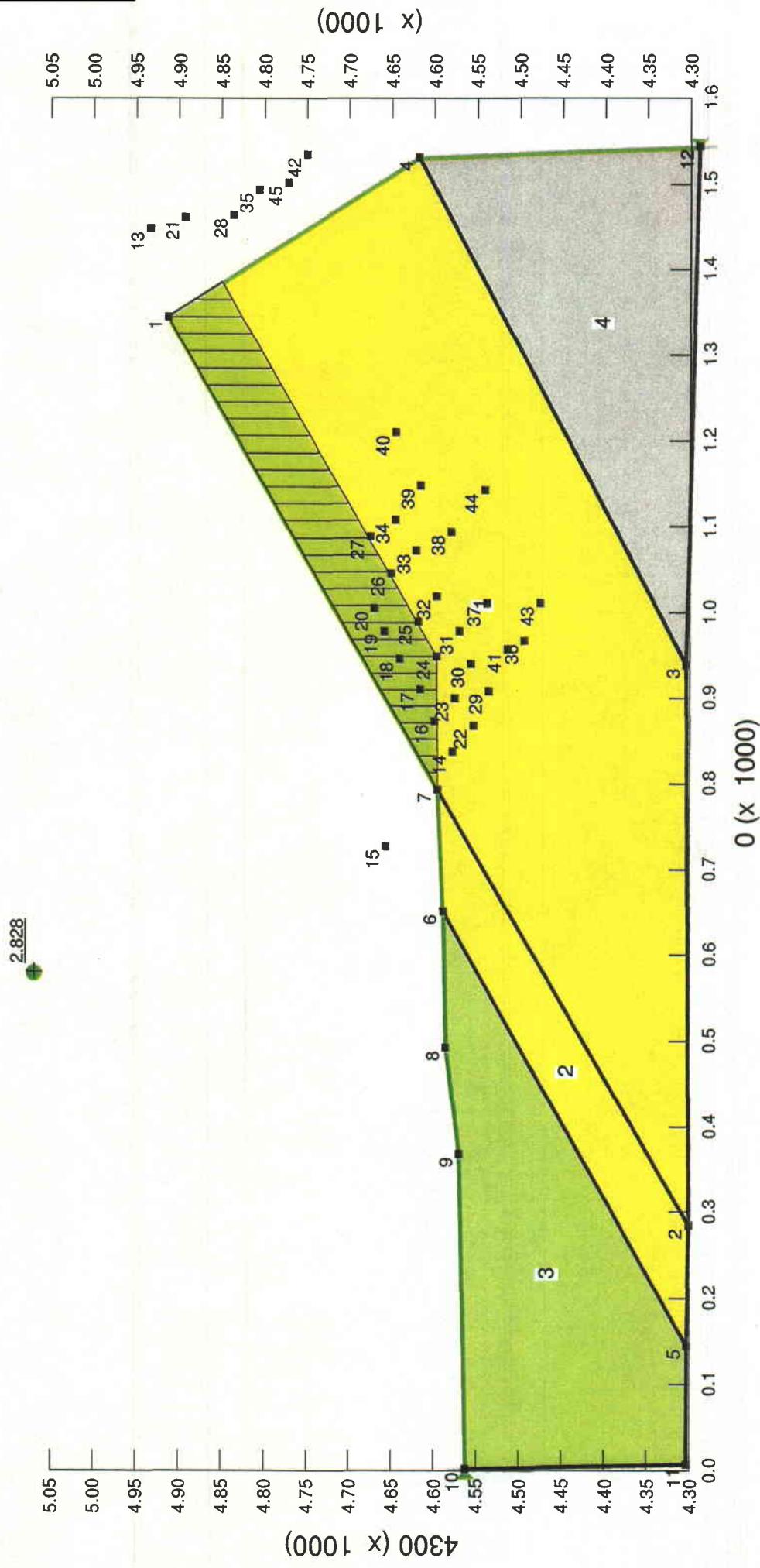
Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



Name: 30 degrees.gsz

Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

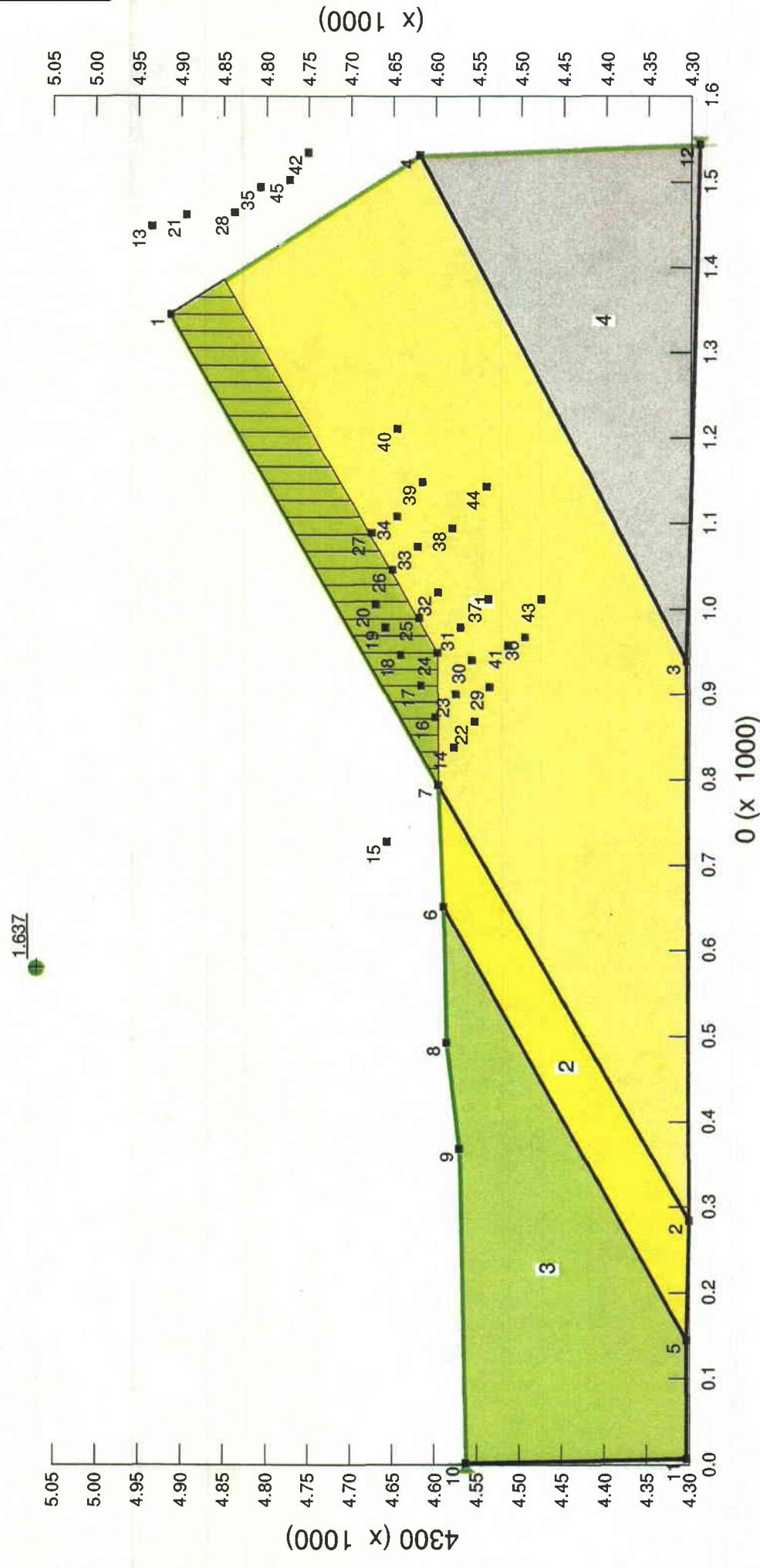
Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



Name: 30 degrees disturbed.gsz

Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

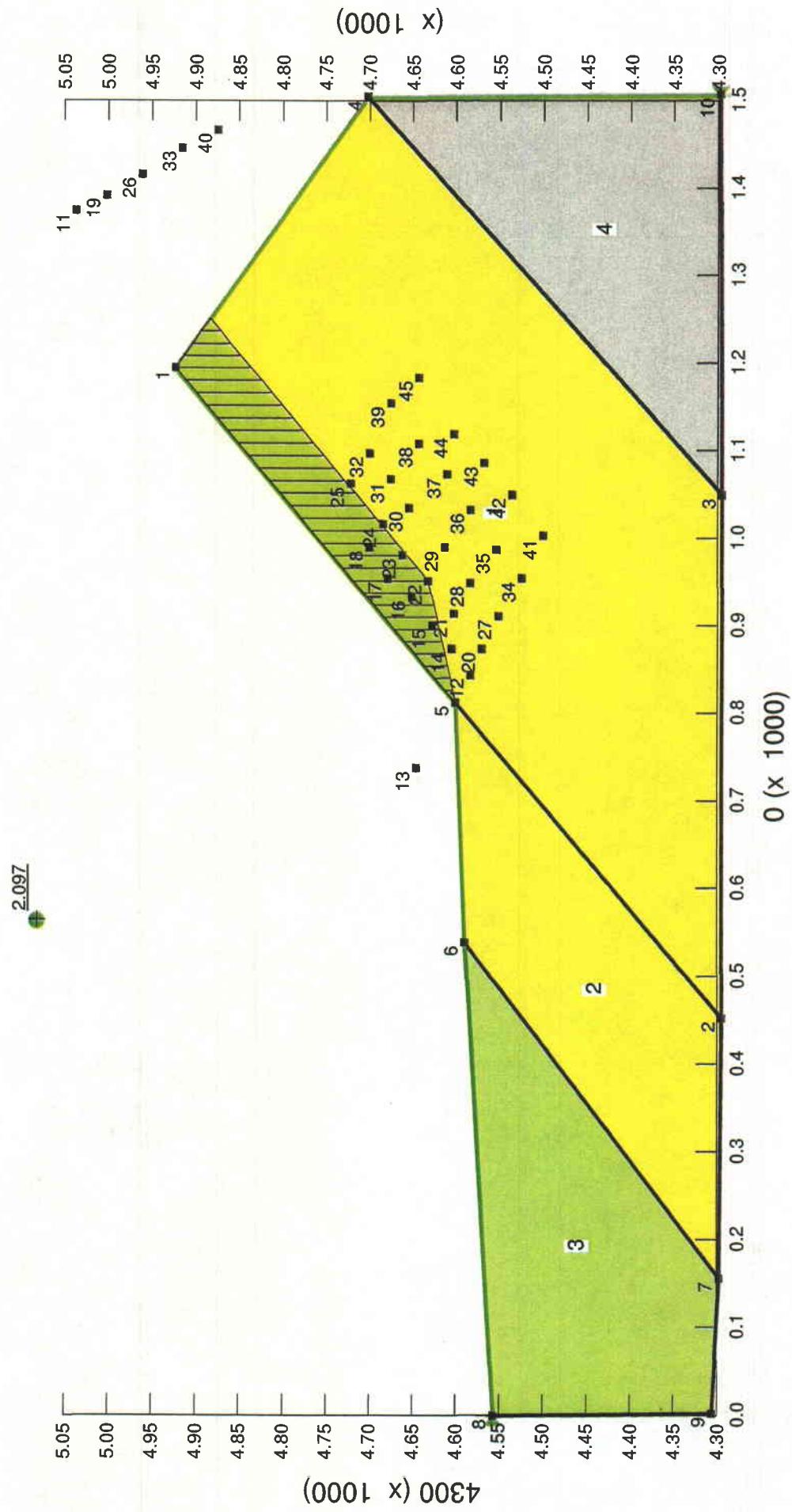
Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



Name: 35 degrees.gsz

Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock



Name: 35 degrees disturbed.gsz

Material #: 1  
Description: Blast Disturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 1  
Material #: 2  
Description: Undisturbed Limestone  
Model: ShearNormalFn  
Wt: 100  
Strength Fn: 2

Material #: 3  
Description: Alluvium  
Model: MohrCoulomb  
Wt: 100  
Cohesion: 0  
Phi: 45  
Material #: 4  
Description: Bedrock  
Model: Bedrock

